

BRIDGE NO.04180

52980 - NEWTOWN INTERSTATE 84 WESTBOUND over HOUSATONIC RIVER

Fracture Critical and Routine Inspection 9/12/2018 Inspected by: Infrastructure



TABLE OF CONTENTS

Section	Page Number
Report Title Page	1
Location Map	2
Structure Inventory and Appraisal (BRI-19)	3
Inspection Data (BRI-18)	7
National Bridge Elements	18
Fracture Critical Data (BRI-12)	20
Rocker Bearing Measurements (BRI-15)	22
Sketches	26
Photos	64



Report Title Page

Project No.:170-3413 (Routine Inspections – NHS)Structure:04180, Interstate 84 Westbound over Housatonic River, NewtownInspected By:Infrastructure Engineers, Inc.Inspection Date:

Professional Certification:

I hereby certify that this report, including all of its contents, has been approved by me, and that I am a duly licensed professional engineer under the laws of the State of Connecticut.



Jay Messier, PE (Infrastructure Engineers, Inc.)

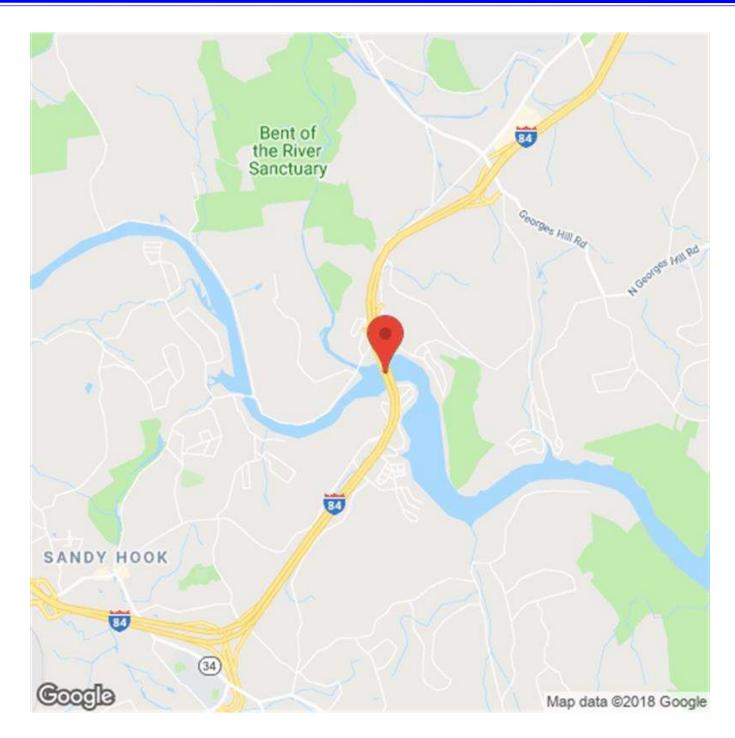
PM / QAQC: Jay Messier, PE

CT License No.: 19023

Date: 10/2/2018

Bridge No: 04180

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Location Map # 1

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

STRUCTURE INVENTORY & APPRAISAL

INSPECTION	STRUCTURE TYPE & MATERIALS
Structurally Deficient N Functionally Obsolete N	(43) Structure Type, Main
Sufficiency Rating 80.7	A) Material 4 - Steel continuous
(90) Inspection Date 09/12/2018 (91) Frequency 24	B) Design Type 03 - Girder and Floorbeam System
Indepth Insp No Proposed next Indepth Year	(44) Structure Type, Approach
Deck Survey Date Class 03	A) Material 0 - Other
Access 15 - Over 40 ft.reach Flagman 0	B) Design Type 00 - Other
Frequency Date Type	(45) Number of Spans, Main Unit 4
Fracture 24 09/12/2018 C Two Girder System, welded plate girders	(46) Number of Approach Spans 0
Underwater 24 B Underwater Only	(107) Deck Structure Type 1 - Concrete Cast-in-Place
Special	(108) Wearing Surface/Protection Systems
IDENTIFICATION	A) Type of Wearing Surface 6 - Bituminous
Bridge Name ROCHAMBEAU BRIDGE	
Town Code - Name 52980 - NEWTOWN	B) Type of Membrane 2 - Preformed Fabric
(5) Inventory Route	C) Type of Deck Protection 0 - None
(A) Record Type 1: Route carried "on" the structure	Substructure
(B) Signing Prefix 1 - INTERSTATE HIGHWAY	A) Material 2 - CONCRETE
(C) Level of Service 1 - MAINLINE	B) Design Type 1 - FULL HEIGHT STEM
(D) Route Number. 00084	Paint
(E) Dir Suffix 4 - WEST	Type 3 - Non-Lead Paint
(6A) Featured Intersected HOUSATONIC RIVER	Year
(6B) Critical Facility Indicator	Comment
(7) Facility Carried INTERSTATE 84 WESTBOUND	GEOMETRIC DATA
(9) Location 1.5 MI EAST OF EXIT 11	(48) Length of Maximum Span 224 ft.
(11) Mile Post 18.42 Miles	(49) Structure Length 792 ft.
(16) Latitude 41 Deg. 26 Min. 21 Sec.	(50) Curb or Sidewalk Widths
(17) Longitude -73 Deg. 14 Min. 53 Sec.	A) Left 0 ft. 0 in. B) Right 0 ft. 0 in.
(98) Border Bridge	(51) Bridge Roadway Width Curb to Curb 60 ft. 0 in.
(A) State Code (B) Percent Responsibility %	(52) Deck Width, Out to Out 63 ft. 9 in.
(C) Border Town Name	(32) Approach Roadway Width 50 ft.
(99) Border Bridge Structure No.	

Form: BRI-19, Rev. 2/15 Inspection type: Fracture Critical,Routine Inspection Date: 9/12/2018 Inspected by: Infrastructure Engineers

Bridge No: 04180

(33) Bridge Median	0 - No median		AGE AND SERVICE
Deck Area 50529	sq. ft.	Year Built 19	77 (106) Year Reconstructed
(34) Skew Angle	deg.	(42) Type of Servi	ce
(35) Structure Flared 0 - No fla	-	A) On 1	- Highway
(10) Inv. Rte. Min. Vert. Clearance	99 ft. 99 in.	B) Under 5 -	Waterway
(47) Inv. Rte. Total Horiz. Clr.	60 ft. 0 in.	(28) Number of La	nes
Log Inv. Rte. Total Horiz. Clr.	60 ft. 0 in.	A) On	02 B) Under 00
RLog Inv. Rte. Total Horiz. Clr.	0 ft. 0 in.	(29) Average Daily	/ Traffic 39795
(53) Min. Vert. Clearence Over Bridge	e 99 ft. 99 in.	Is Above Half A	DT? Yes
(54) Log-Min. Vert. Underclearance	N ref. 0 ft. 0 in.	(109) Precent T	ruck 9 %
(55) Min. Lat Underclearance on Righ	nt N ref. 0 ft. 0 in.	(30) Years of A	DT 2018
(56) Min. Lat Underclearance on Left	0 ft. 0 in.	(19) Bypass, Deto	ur Length 1 Miles
CONDITI	ON		APPRAISALS
(58) Deck	5	(67) Structural Eva	luation 5
(59) Superstructure	5	(68) Deck Geometr	ry 9
(60) Substructure	6	(69) Underclearanc	ces, Vert. & Horiz. N
(61) Channel & Channel Protections	5	(71) Waterway Ade	equacy 9
(62) Culverts	Ν	(72) Approach Roa	idway Alignment 8
(36) Traffic Safety Features		(113) Scour Critica	I 8
A) Bridge Railings	0		<u>COMMENTS</u>
B) Transitions	0	Special inspectio main girders.	n: Horizontal cracking along floorbeams &
C) Approach Guardrail	1		
D) Approach Guardrail En	ds 1		
WATER			- CLASSIFICATION
Drainage Basin Waterway	6000 - Housatonic River	(112) NBIS Bridge L	Length Yes
	0 - No navigation control on waterway (bridge permit not required)	(104) Highway Syst	em 1 - Structure/Route is on NHS
(39) Navigation Vertical Clearance	0 ft.	(26) Functional Clas	ss 01 - Rural - Principal Arterial - Interstate
(40) Navigation Horiz. Clr.	0 ft.	(100) Defense High	way 1 - Is on an Interstate STRAHNET route
(111) Pier/Abutment Navigation	2 - In place and functioning	(101) Parallel Struct	L - Left structure (South or West)
(116) Vert-Lift Brg Nav Min	0 ft. 0 In.	(102) Direction of T	raffic 1 - 1-way traffic

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND **Crossed:** HOUSATONIC RIVER Inventory Route: NHS

(103) Temporary Stru	cture					
		1 - Inve Networl	nventory route on National Truck vork			
(20) Toll		3 - On F	3 - On Free Road			
(21) Maintain		01 - Sta	ite High	way	Agency	
(22) Owner		01 - Sta	ite High	way	Agency	
Report Class		S - STA	TE			
(37) Historical Signific	ance	5 - Not	eligible	for N	ational Register	
	— РС	STED	SIGNS	; —		
Other Posted Sign 1						
Other Posted Sign 2						
			Actual		Recomended	
Posted Load Single Unit Tru		ck				tons
Posted Load Semi-Trailer Tru		ruck				tons
Posted Load 4 Axle Truck						tons
Posted Load 3S2 Truck						tons
All Vehicles						tons
Posted Vert. Clearance on Brid		Bridge]ft. [in.	
Posted Vert. Undercle	earance	Э]ft. [in.	
Posted Speed Limit of	on Bridg	je]m.p	.h.	
	- отн	ER FE	TURE	S –		
Fence Required	No					
Fence Present	No					
Fence Type	Blank					
Fence Height						
Fence Material	Blank					
Fence Top Type	Blank					
Barrel Ladders	Yes					
Stand Pipes	No					
Catwalks	Yes					
Moveable Inspection	System	1	No			
Haunches Present ov	er Roa	dway	NO			
Utilities	N No	Utilities	present	t		

PROPOSED IMPR	OVEMENTS
(75A) Type of Work Proposed	35 - Rehabilitation -
(75B) Work Done By	Deterioration 1 - Work to be done by
(76) Length of Structure Improvement	contractft.
(94) Bridge Improvement Cost	\$
(95) Roadway Improvement Cost	\$
(96) Total Project Cost	\$ 30000
(97) Year of Improvement Estimate	2014
(114) Future ADT	59131
(115) Year of Future ADT	2038
DOT Bridge Program List No	27
Project No	0096-0201

- LOAD RATING & POSTING ·

A - Open

09/18/2019

(31) Design Load	5 - HS 20
(63) Operating Rating Type	1 - Load Factor (LF)
(64) Operating Rating	60
(65) Inventory Rating Type	1 - Load Factor (LF)
(66) Inventory Rating	36
Evaluation Code	L - Load Factor
Year of Evaluation	1995
(70) Bridge Posting	5 - Equal to or above legal loads

(41) Structure Status

Advertised Date

INSPECTOR'S SIGNATURES:

1)	Michel O'Han	Date:	10/01/2018	P.E. SIGNATURE:	I Massin	Date:	10/04/2018
2)	Macree figuore	Date:	10/01/2018	P.E. #		– – Date:	10/23/2018
3)	Matthew Rusenburg	Date:	10/02/2018		AmyEstaba	_	
4)		Date:					

		<u>FII</u>	ELD INSF	PECTIO	ON REP	<u>ORT</u>			
Location:	1.5 MI EAST OF	= EXIT 11	Year Built:	1977		Snoop	er Required:		
Main Material:	4 - Steel continu		Year Rebuilt			•	er Used:		
Main Design:	03 - Girder and	Floorbeam	-					_	
			_						
Inspectors:					Visits:				
Lead Inspector	:	Matt R	iesenberg		Visit Date:	Temp:	Start Time:	End Time:	
Inspector:		Task:			09/12/2018	75	08:30 AM	03:30 PM	
Area, 12		BSE -	Manager		09/13/2018	70	08:00 AM	01:00 PM	
Liguore, Matt			Inspector						
O'hara, Mike			nspector Inspector		_				
Riesenberg, Ma	att		Inspector		_				
		201			_				
58. DECK:									
Rei	inforced Concrete	e Deck with Bitu	minous Concre	te Overlay				Overall Rating	j: 5
	Rating Overlay: 5	Bituminous cor							
Dock	Str. Condition: 5	 Light wear with Random short Up to 1/4" dee Bituminous pathematic Paving seams Span 1 adjact cracks. Span 2 has m 3-1/2" deep, or (See Sketches) The underside 	t cracks open u ep milled areas atches in the sh s open up to 3" ent to Abutmen nultiple potholes he isolated poth 2 - 6 and Photo	up to 1/8" w along old wide wors wide wors at 1 has two s / failed pa hole has ex os 7 - 9)	vide. shifted lane li o to 20' long x t case in Spa o (2) 20' long t atches, mostly posed deck.	nes. 5' wide. n 2. x 3' wide	x open up to		as of map ong x 10" wide x
		 Random area Random hairl Hairline map Areas of mois 	s of light scalin ine Transverse cracks up to 10 sture throughou or Beam 16 and w areas up to 2 een Floor Beam	g. cracks wit 00% of the l it. 1 17 in Bay 2' long x 1' y 1 21 & 22 in	h and without bays with and 1 - there is a wide. h Bay 4 has a	l without 8' x 8' m n isolated	efflorescence oderate scale d 6" diameter	e with efflores	cence and a 1' x pall.
	Curbs: 7	Sloped granite	curbs along bo	oth sides of	the bridge ha	ave the fo	llowing defici	iencies:	
		Minor scrapes Random spal Average Curb Left (West): 3-3 Right (East): 3-3	ls up to 3-1/2' lo Reveal: 3/4"		gh x 1-1/2" de	ep.			

ed by: Infrastructure	Engineers Inventory Route: NHS
	• The approach curb at the southwest corner is laterally misaligned up to 6" over a 12' long section.
	(See Sketches 2 - 6 and Photos 12 & 13)
Median: N	
Sidewalks: N	
Parapet: 6	Reinforced concrete parapets have the following deficiencies:
	 Random areas of scale up to 13' long x 1' wide x 1" deep. Spalls up to 18" long x 10" wide x 2" deep with and without exposed rebar. Some spalls undermine the railing post bases and have exposed anchor bolts. Random horizontal and full height vertical cracks open up to 1/8" wide. Random hollow areas up to 28" long x up to 1' wide.
Railing: 6	(See Sketches 2 - 6 and Photos 13 - 15) Single aluminum railings on both parapets have the following deficiencies:
	 Random collision scrapes. Random loose anchor bolt nuts. Right Railing in the South Approach has two rail posts (4th & 6th) with (1) of (4) missing anchor bolt nut. Right Railing in the South Approach end angle cracked at 135 degree angle weld and at base plate weld. Left Railing in Span 3 has isolated posts with exposed anchor bolts due to parapet spalls or scaling. Left Railing in Span 4 has minor gouges from impact damage and (2) posts with up to (1) of (4) sheared off anchor bolts. Left Railing in Span 4 has a random bent base plate and (1) post has a 2-1/2" long crack in the stanchion due to previous impact damage (no change). (See Sketches 2 - 6 and Photos 13, 16 - 18)
Paint: N	
Fence: N	
Drains: 5	Scupper grates and drains along both sides of the bridge have the following deficiencies:
	 Grates are partially clogged up to 50% with sand and vegetation, but pipes are clear. There are random scupper supports with heavy laminar rust and u-bolts with up to 60% section loss. There is a depressed area in the Right Curb at Abutment 2 between the scupper grate and joint where water is ponding with leakage at the joint below. PVC weep pipes at each floor beam along both overhangs have the following deficiencies:
	 Weep pipes are mostly short but point away from the superstructure however, approximately 50% of all weep pipe locations show evidence of past drainage on the superstructure. Several weep drains are short and are leaking onto the floor beam below. Span 2, weep pipe between Stringers 7 & 8 at south side of Floor Beam 15 is short and has heavy efflorescence. No evidence of leakage on to steel was noted.
Lighting Ctondord 0	(See Sketches 2 - 7, 9, 10, 12 - 16, 18, 20 - 23, 25 & 29 and Photos 19 - 22)
Lighting Standard: 6	 The electrical conduits for lighting standards are not capped. Junction box covers along the Left Parapet are missing up to (9) of (10) screws but are secure. The attached sign on the Left Parapet in Span 2 has the following deficiency: Impact damage.
	The navigation lights were not on at the time of inspection.

Form: BRI-18, Rev. 1/14 Inspection type: Fracture Critical,Routine Inspection Date: 9/12/2018 Inspected by: Infrastructure Engineers

Bridge No: 04180

	(See Sketches 2 - 6 and Photo 23)		
Overall Utility Condition Rat	ing N - Not Applicable		
Utility Type/Size			
N N(DUtilities present		
Construction Joints:	8		
Expansion Joint:	Expansion Joint: 6 Strip seal joints at both abutments have the following deficiencies:		
	 Light to heavy accumulation of sand and debris (60% of the length), mostly in shoulders. Concrete headers have random full length (about 15" long) transverse cracks spaced 2' to 3' apart. Active joint leakage at both abutments. There is leakage at the Abutment 2 joint at the Right Cantilever. Abutment 1 South Concrete Header has a 1" long x 1' wide crack / spall. Abutment 1 has a pin hole in the joint seal. Abutment 2 North Concrete Header has a 2" long x 6' wide area of deterioration and a 2" long x 3' wide x 2" deep adjacent spall. 		
	The parapet steel joint plates have the following deficiencies:		
	 Left Parapet joint plate cover at Abutment 2 is misaligned up to 1" towards the roadway (See Photo 28). Left Parapet joint plate cover at Abutment 1 is missing (3) of (12) screws (See Photo 26). Right joint plate cover at Abutment 1 has loose screws. 		
	(See Sketches 2, 3 & 6 and Photos 24 - 28)		
Haunches Present over trave	elway? NO		

APPROACH CONDITION:

Bituminous Approac	ch Pavement	Overall Rating: 6
Rating		
Approach Slab: 6	Not visible. Rating is based on the condition of the approach	pavement.
	Refer to 'Approach Pavement' item below.	
Relief Joints: N		
Approach Guide Rail: 6	Metal beam approach guide rails have the following deficience	ies:
	Minor scrapes and dents.	
	 Southeast approach metal beam guide rail at the 4th & 5th p connection bolts. The posts are missing connection bolts to the 	
	(See Sketches 3 & 6 and Photos 29 - 31)	
Approach Pavement: 6	Bituminous approach pavement has the following deficiencies	S:
	• Minor wear.	
	 Paving seams are open up to 1/8" wide. South Approach adjacent to the Abutment 1 concrete header 	er is an area of 1' long x 3' wide breaking up
	bituminous and a 3" diameter x 1" deep pothole.	
	 North Approach Pavement has full width cracks up to 1/8" w has heavy cracking. 	vide and pavement around the joint header
	(See Sketches 3 & 6 and Photos 32 & 33)	
Approach Embankment: 8		

Trafic Safety Features Bridge Railings: 0 Less than 42" high solid concrete parapets on NHS. Transitions: 0 Top of concrete parapet is 3" higher than approach rail.

Transitions: 0	Top of concrete parapet is 3" higher than approach rail.
Approach Guardrails: 1	R-B 350 compliant.
Approach Guardrail Ends: 1	Continuous.

59. SUPERSTRUCTURE:

Girders, Floor Beam	as and Stringers.	Overall Rating: 5
Rating		
Bearing Devices: 6	Rocker Bearings at the Abutments, Pier 1 and Pier 3 have the following	deficiencies:
	 Areas of peeling paint with light to moderate rust. Light to heavy abrasion rust on pins. Up to 1/8" pack rust under the rockers Debris / sand accumulation between the masonry plates and rockers. 	
	Fixed rocker bearings at Pier 2 have the following deficiencies:	
	 Areas of peeling paint with light rust. 	
	Stringer plate / block bearings have the following deficiencies:	
	 Areas of peeling paint with light to moderate rust. Random bearings have shim plates. 	
	• Up to 1/4" gaps between edges of bearing / shim plate and floorbeam rust.	top flange with or without pack
	Abrasion rust and tilted anchor bolts.Moderate to heavy rust on anchor bolts.	
	 Random anchor bolts are slightly tilted to the south or north. 	
	• Span 4 Stringer 8 Bearing at Floor Beam 25 - up to 1/4" deep section faces.	loss on bearing plate external
	(See BRI-15, Sketches 7, 9, 10, 12, - 16, 18, 20 - 23, 25 & 29 and Phot	tos 34 - 39)
Stringers: 5	Stringers have the following deficiencies:	
	 Areas of peeling paint with moderate rust. Isolated areas of heavy rust with and without minor section loss. Some Areas of laminar rust on Stringer 1 and Stringer 8 at the floor beams w Span 1, Stringer 7 between Floor Beam 4 & Floor Beam 5 has a 3" lor Span 4, Stringer 8 at Floor Beam 25 - Left side of web at the bearing h 3/16" deep section loss (<5% section loss for shear & 39.9% section loss underside on both sides of bearing have +/- 1" long x full width x up to 1 x full width x up to 1/8" deep section loss. The top flange of the stringer loss (painted over). (See Photo 42 & 43) 	vith negligible section loss. Ing rolling defect (See Photo 41). Inas a 4' long x 2" high x up to as for bearing). Bottom flange /4" deep section loss and 3' long
	(See Sketches Sketches 7 - 29 and Photos 40 - 45)	
Girders: 5	 Girders have the following deficiencies: Isolated areas of peeling paint with moderate rust, mostly at top flange Random areas of graffiti on the girder webs. Girder web at the floor beam knee brace connections (fascia) have rar 6" high x 6" wide x 3/16" deep resulting in less than 5% web loss. Isolated web stiffeners have up to full width x 1/8" deep section loss at Random bowed / warped webs. Bottom flanges have isolated areas pitting up to 25' long x up to 10" w 	ndom areas of section loss up to

Form: BRI-18, Rev. 1/14 Inspection type: Fracture Critical,Routine Inspection Date: 9/12/2018 Inspected by: Infrastructure Engineers

	than 1% flange loss. • Random areas of bent bottom flanges and top flanges +/- 1/4" (appear as-built) (no change).
	Refer to 'Welds-Cracks' item below.
	(See Sketches 7 - 29 and Photos 46 - 48)
Floor Beams: 5	Floor Beams and Floor Beam Cantilevers have the following deficiencies:
	 Random areas of peeling paint with up to moderate rust. Random areas of painted over section loss have reactivating rust. Random bowed / warped webs up to 3/4". (See Photos 53, 56, 57, 62 & 65) Top flange of floor beams at cantilever ends have section loss up to full width x 1/8" deep resulting in up to 9% flange loss (See Photo 67).
	Refer to 'Welds-Cracks' item below.
	(See Sketches 7 - 31 and Photos 49 - 70)
Trusses - General: N	
Trusses - Portals: N	
Trusses - Bracing: N	
Paint: 7	Less than 10% of painted surfaces have deterioration.
Rust: 6	Refer to items above.
Machinery Movable Span: N	
Rivets & Bolts: 6	Connection bolts throughout the bridge have the following deficiencies:
	 Random areas of light to heavy rust.
	Connection bolts at floor beam tie plates:
	 Isolated floorbeam top flange tie plates have heavy rust with negligible section loss on the bolts. Floor Beam 19 top flange tie plate bolted connection at G1 and G2 in Span 3 each has 1 (of 64) loose bolt.
	Girder and Stringer Bolted Field Splices:
	 All bolted field splices were checked 100% hands-on.
	 Span 2, Girder 1 top flange splice near Floor Beam 8 has (1) loose bolt. Span 3, Girder 1 bottom flange splice near Floor Beam 22 has (1) loose bolt.
	 Span 3, Grider 1 bottom hange spice hear noor beam 22 mas (1) loose bott. Stringer splices have isolated areas of pack rust up to 1/4" between the splice plates and the stringer
	webs / flanges. (See Photo 40) • Stringer diaphragms at Floor Beam 29 have bottom two (2) bolts missing (See Photo 45).
	(See Sketches 7 - 29 and Photos 40 & 45)
Welds - Cracks: 5	 There are poor quality welds with and without undercuts (+/-1/8" deep) throughout the structure. Girder flange transition welds were inspected 100% hands-on and have isolated light rust.
	 Girder Cracks: Span 1, south side of Floor Beam 4 at Girder 1, floor beam-to-girder connection plate weld at the top has a stop hole with a 1/2" long crack (no change). Span 4, east side of Girder 2 web at Floor Beam 25 below cope hole for floor beam tie plate has a 1/8" long vertical crack with a 3/4" diameter stop hole (no change). There is a 14" long broken weld between the top of the G1 top flange and diaphragm vertical tie plate at Abutment 1 (no change). (See Photo 51). Random girder flange splice plates are tack welded at the vertical edges with no evidence of cracking.

	Floor Beam Cracks:
	 Floor Beam Cracks: Floor beam and floor beam cantilevers have numerous horizontal cracks in the toe of the fillet weld between the floor beam top flanges and webs. The floor beams are retrofitted with stop holes (typically 3/4" diameter) to prevent crack propagation and a portion of the girder-to-floor beam connection plates have been removed at a majority of crack locations. (See Photos 52, 54, 58 - 61, 64, 68 - 70) A majority of locations which were previously noted as cracks were either peeling/cracked paint with or without rust, or, undercutting along the welds. (Comment from 2016 Report) Isolated crack lengths have been changed since the previous inspection however, a majority of the crack lengths remain unchanged since the previous inspection. Maximum crack length found was at 17-1/2" long at Floor beam top flange tie plates are tack welded to the girder webs at the cope holes and isolated floor beam web stiffeners are tack welded to the floor beam top flanges. Some of these tack welds are broken and have abrasion dust emanating from them. Broken tack welds were found at the following locations: FB 3 north side web stiffener to top flange at S7. FB 4 top flange tie plate at G1. FB 7 south side web stiffener to top flange at S6. FB 21 south side web stiffener to top flange at S6. FB 21 south side web stiffener to top flange at S6. FB 26 top flange tie plate at G1. A several locations, the horizontal face of the flame cut girder-to-floor beam connection plate is partially fused to the floor beam connection plate is partially fused to the flame cut girder-to-floor beam connection plate is partially fused to the floor beam connection plate is partially fused to the floor beam connection plate is partially fused to the floor beam connection plate is partially fused to the floor beam connection plate is partially fused to the floor beam webs because of the flame cutting.
	(See Sketches 7 - 29 and Photos 52, 54, 58 - 61, 64, 68, 69 & 70)
Timber Decay: N	
Concrete Cracking: N	
Collision Damage: 7	 Minor dents in the bottom flange of Stringer 8 between Floor Beam 14 and Floor Beam 15 (appear to be construction related). 4" x 1/2" x 5/16" deep dent in Stringer 8 between Floor Beam 26 and Floor Beam 27 (appear to be construction related). (See Photo 44)
Member Alignment: 5	Floor beam and floor beam cantilevers frame into the girders perpendicularly with bolted connections. The girder-to-floor beam connection plates are welded to the girder webs and bolted to the floor beam webs.
	 Several bolted connections between the girders and the floor beam have bowed / warped webs on the girders and floor beams up to full height of the floor beam web. The most severe bow was noted at the east side of Girder 1 bolted connection to Floor Beam 18 in Span 3. At this location, Floor Beam 18 Web is warped up to full height x +/-3/4" out of plane (no change since the last inspection). Close monitoring of these conditions is recommended. There are random gaps between the girder-to-floor beam connection plates and the floor beam webs up to 3/4"; worst at the top of the connection plates and tapering to 0" at the bottom of the connection plates. Several floor beam top flange tie plates are in contact with the girder webs at the cope hole. Random
	 Several noor beam top hange the plates are in contact with the girder webs at the cope hole. Nandom locations have abrasion rust. (See Photos 55 & 63) There are isolated locations at the stringer bottom flanges that are vertically misaligned up to 3/16" with or without pack rust at the bolted field splices (no change this inspection) with isolated bent bottom flange splice plates at Stringer 6 near Floor Beam 19 (See Photo 40)
Deflection Under Load: N	(N) - Normal; (E) - Excessive
Vibration Under Load: N	(N) - Normal; (E) - Excessive

Form: BRI-18, Rev. 1/14 Inspection type: Fracture Critical,Routine Inspection Date: 9/12/2018 Inspected by: Infrastructure Engineers

Bridge No: 04180

Stand Pipes:	
Catwalks:	• Catwalk between Stringer 4 & 5 has areas of peeling paint and light to moderate rust.
Movable Inspection System:	
Barrel Ladders:	N I I I I I I I I I I I I I I I I I I I
	Are Barrel Ladders OSHA Compliant? NA

60. SUBSTRUCTURE:

Reinforced concrete	abutments, wingwalls and piers.	Overall Rating: 6
Rating		
Abutments - Stem: 6	The abutment stems are significantly covered with	fill material. The following deficiencies were noted.
	diameter hollow area. • Abutment 2 Cheekwall adjacent to Girder 2 has a	" wide x 3/4" deep spall. face up to 1' wide x full height x up to 2-1/2" deep 1' high x 8" wide x 1-1/2" deep spall with adjacent 8"
	(See Sketches 32 & 33 and Photos 71 - 74)	
Abutments - Backwall: 5	Abutment backwalls have the following deficiencies	3:
	exposed rebar. • Abutment 2 has an isolated 34" high x 19" wide he	xposed rebar. /alls. " deep concrete cut out / spall. up to 3-1/2" deep concrete cut out / spall with two (2)
· · · · · · ·	(See Sketches 32 & 33 and Photos 71, 74 & 75)	
Abutments - Footings: N	Not visible.	
Abutments - Settlement: 8	None noted.	
Abutments - Wingwalls: 7	 Wingwalls have the following deficiencies: Random small pop-outs. Moderate to heavy vegetation growth. Missing 100% of filler material at the joints. Wingwall 1B fence has a +/- 10' long section that Wingwall 2B has in isolated 1' diagonal crack. (See Sketches 34 & 35 and Photo 76) 	is damaged / missing.
Piers/Bents - Caps: N		
Piers/Bents - Pile Bent: N		
Piers/Bents - Columns: 6	Solid concrete pier walls have the following deficier	ncies:
	 Pedestals have hollow areas up to full length x 1' Pop-outs up to 6" diameter x 1/2" deep. Random vertical hairline cracks up to 6' long with Pier 2 - Girder 1 Pedestal has two spalls up to 1' I 	

	 bearing). Pier 2 - Girder 2 Pedestal has two spalls up to 1' long x full width x full height (does not undermine the bearing). Pier 3 - South Elevation has areas of hairline map cracks up to 2' high x 2' wide. Per Underwater Inspection report by Infrastructure dated 07/18/2018: Pier 1 has a 5' high band of abrasion up to 1/4" deep extending 1' above to 4' below the water surface. There are cracks up to 1/2" wide along the cold joints above and below water and isolated vertical and horizontal cracks up to 1/16" wide on the south face of the pier. Pier 2 has a 5' high band of abrasion up to 1/4" deep extending from 1' above to 4' below the water surface. There are steel H-piles intermittently exposed along the stem located above the step footing (form work left in place from construction). Pier 3 has a 5' high band of abrasion up to 1/4" deep extending from 1' above the water surface to the channel bottom. The north face towards the upstream (west) nose of the pier has a 1.7' long vertical
	hairline crack below the construction joint.
	(See Sketches 36 -38 and Photos 77 & 78)
Piers/Bents - Footings: 6	See item "Erosion-Scour" below.
Piers/Bents - Settlement: 8	None noted.
Erosion - Scour: 5	 Erosion - Rating = 6. The embankment along Wingwall 1A has 10' long x 3' wide x up to 1' deep erosion (footing not exposed). Previously noted erosion along Wingwall 2A was not found.
	 Scour - Rating = 5. Per Underwater Inspection report by Infrastructure dated 07/18/2018: Pier 1: Step footing is exposed over its full length x up to full height (5.4' high) along both faces of the pier. The footing is exposed up to 5 and the footing has been exposed up to 3.9' vertically over a length of 37' on both faces of the pier (footing was exposed up to 3.9' high per the 2016 underwater inspection). Channel elevations have varied around the pier with an increase by up to 1.9', this is possibly due to the timber surrounding the pier. Along the north face of the pier, there is a 20' length of sheet pile exposure up to 6' high located 5' off the pier face.
	 Pier 2: The step footing is exposed over its full-length x up to full-height (5.0' high) on both faces of the pier. The footing has been exposed full length x up to full-height (5.5' high) on the south face and full length x up to 1.7' high (previously 0.2' high) on the north face of the pier with no undermining. The tremie seal has been exposed up to 1.0' long across the full-width of the Upstream (West) Nose and over 2' long x 0.5' high at the southwest corner of the pier. Random vertical hairline cracks up to 3' long with efflorescence on the footing. Isolated edge spalls on the footing up to 2' long x 4" wide x up to 4" deep. At the center of the Downstream (South) face of the step footing there is a spall 18" long x 1' wide x 4" deep. Channel elevations surrounding Pier 2 has had aggradation of up to 3.1' with isolated areas of degradation up to 3.0'. The Tremie Seal has been exposed up to 1.0' high (previously 0.9' high) across the full-width of the Upstream (West) Nose and over 2' long x 0.5' high at the southwest corner of the pier.
	 Pier 3: There is a section of sheet pile exposed up to 6" high along the south face of the pier approximately 12.5' off the pier face. Channel bottom elevations have remained relatively unchanged (less than 0.6' variations) except for scour up to 3.0' along the north face of the pier since 2016 Inspection.

	(See 2018 Underwater Report, Sketch 34 & 35 and Photos 79 & 80)
Concrete Crack - Spall: 5	See items above.
Steel Corrosion: N	
Paint: N	
Timber Decay: N	
Collision Damage: 8	
Debris: 8	
61. CHANNEL AND CHANNEL	_ PROTECTION:
Housatonic River	Overall Rating: 5
Rating	
Channel - Scour: 5	Per Underwater Inspection report by Infrastructure dated 07/18/2018:
	 Channel bottom elevations along the upstream and downstream fascias have random areas of mostly aggradation up to 4.0' since the 2016 Underwater Inspection. Channel bottom elevations at Pier 1 have varied around the pier with an increase by up to 1.9', this is possibly due to the timber surrounding the pier since the 2016 Underwater Inspection. Channel bottom elevations at Pier 2 show evidence of aggradation of up to 3.1' with isolated areas of degradation up to 3.0' since the 2016 Underwater Inspection. Channel bottom elevations at Pier 3 south face have remained relatively unchanged (less than 0.6' variations) as compared to the 2016 Underwater Inspections. Channel bottom elevations at Pier 3 show evidence of degradation of up to 3.0' along the north face of the pier most likely due to the sandbar that was noted in the previous 2016 Underwater Inspection. At Pier 1 and 2, the step footing and footing have been exposed due to scour. At Pier 1, the step footing is exposed over its full-length x up to full-height (5.4' high) along both faces of the pier, and the footing has been exposed up to 5.1' vertically (previously 5.0' high) on the south face of the pier and the footing has been exposed over its full-length x up to full-height (5.0' high) on the south face and full length x up to 1.7' high (previously 0.2' high) on the north face of the pier. At Pier 2 the Tremie Seal has been exposed over its full-length x up to full-height (5.5' high) on the south face and full length x up to 1.7' high (previously 0.2' high) on the north face of the pier. At Pier 2 the Tremie Seal has been exposed up to 1.0' high (previously 0.9') across the full-width of the upstream (west) nose and over 2' long x up to 0.5' high at the southwest corner of the pier. At Pier 2 the Tremie Seal has been exposed up to 1.0' high (previously 0.9') across the full-width of the upstream (west) nose and over 2' long x up to 0.5' high at the southwest corner
Embankment - Erosion: 7	There is no significant erosion along the channel embankments.
	(See 2018 Underwater Inspection Report)
Debris: 6	Per Underwater Inspection report by Infrastructure dated 07/18/2018:
	 There is timber debris up to 1' diameter at the upstream (west) and downstream (east) noses of Pier 1. There is moderate timber debris up to 2' diameter along the north face of Pier 1. There is timber debris up to 1' diameter at the upstream (west) nose of Pier 2. (See 2018 Underwater Inspection Report)
Vegetation: 7	• The channel embankments are well vegetated.
	(See 2018 Underwater Inspection Report)
Channel Change: 6	Per Underwater Inspection report by Infrastructure dated 07/18/2018:
	 There is no apparent change to the channel orientation.

• Bridge No. 01218 is located approximately 40' downstream (East) of the bridge.

• There is approximately a 150' long x 20' wide x 3' high sandbar/area of aggradation located north of

Overall Rating: N

	Pier 3.
	(See 2018 Underwater Inspection Report)
Fender - System: N	• The navigation lights on Piers 1 and 2 and the west bridge fascia appear to be on timers and were not on at the time of inspection.
Spur Dikes and Jetties: N	
Rip Rap: N	

62. CULVERTS AND RETAINING WALLS:

Rating	
Barrel: N	
Concrete: N	
Steel: N	
Timber: N	
Headwall: N	
Cutoff Wall: N	
Debris: N	
Retaining Wall System: N	
Footing: N	

LOAD POSTING:

ng

Single Unit (Tons):	
Semi Trailer (Tons):	
4 Axle (Tons):	
3S2 (Tons):	
All Vechicles:	
Advanced Warning:	
Warning At Bridge:	
Legibility:	
Visibility:	

VERTICAL CLEARANCE POSTING

Min. Vert Under Clearance:	Ft	In			
Posted Clearence Under Bridge:	Ft	In			
Posted Clearence On Bridge:	Ft	In			
Advanced Warning:					
Warning At Bridge:					
Legibility:					
Visibility:					

NOTES / COMMENTS:

Character of Traffic: Heavy volume of mixed weight traffic.

Additional Notes:

- Bridge Identification Number is clear and legible.

Form: BRI-18, Rev. 1/14						
Inspection type:	Fracture Critical, Routine					
Inspection Date:	9/12/2018					
Inspected by: Inf	rastructure Engineers					

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

- Bridge is logged from west to east, but items are labeled south to north with Girder 1 and Stringer 1 at the Left (West) fascia which is consistent with previous reports and bridge plans.

- Bridge was inspected using catwalk, 60' snooper, rope access, safety boat, trooper and shoulder closures on I-84 Eastbound.

Additional Comments:

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

National Bridge Elements Inspection type: Fracture Critical,Routine Inspection Date: 9/12/2018 Inspected by: Infrastructure Engineers

	Environment	Total Quantity	Units	Condition State 1	Condition State 2	Condition State 3	Condition State 4
12 - Reinforced Concrete Deck	Mod.	50529	sq. ft.	45900	4623	6	0
1080 - Delamination/Spall/Patched Area	l	9		0	3	6	0
1090 - Exposed Rebar		2		0	2	0	0
1120 - Efflorescence/Rust Staining		2877		0	2877	0	0
1130 - Cracking (RC and Other)		1741		0	1741	0	0
510 - Wearing Surfaces		47520	sq. ft.	47367	10	143	0
3210 - Delamination/Spall/Patched Area/Pothole (Wearing Surfaces)		13		0	10	3	0
3220 - Crack (Wearing Surface)		140		0	0	140	0
107 - Steel Open Girder/Beam	Mod.	1584	ft.	1453	96	35	0
1000 - Corrosion		126		0	91	35	0
1900 - Distortion	1	5		0	5	0	0
515 - Steel Protective Coating		62656	sq. ft.	59531	2500	625	0
113 - Steel Stringer	Mod.	6336	ft.	5991	334	11	0
1000 - Corrosion		339		0	329	10	0
1900 - Distortion	1	5		0	5	0	0
7000 - Damage	;	1		0	0	1	0
515 - Steel Protective Coating		44352	sq. ft.	42852	1400	100	0
152 - Steel Floor Beam	Mod.	1850	ft.	1603	213	34	0
1000 - Corrosion		157		0	135	22	0
1010 - Cracking		79		0	67	12	0
1900 - Distortion	1	11		0	11	0	0
515 - Steel Protective Coating		28870	sq. ft.	28870	0	0	0
210 - Reinforced Concrete Pier Wall	Mod.	151	ft.	9	42	100	0
1080 - Delamination/Spall/Patched Area	l	16		0	16	0	0
1130 - Cracking (RC and Other)		26		0	26	0	0
6000 - Scour	-	100		0	0	100	0
215 - Reinforced Concrete Abutment	Mod.	128	ft.	116	0	12	0
1080 - Delamination/Spall/Patched Area		12		0	0	12	0
300 - Strip Seal Expansion Joint	Mod.	120	ft.	70	46	4	0
2310 - Leakage		1		0	0	1	0
2350 - Debris Impaction	1	24		0	24	0	0
2360 - Adjacent Deck or Header		25		0	22	3	0
311 - Movable Bearing		8	each	2	6	0	0
1000 - Corrosion		6		0	6	0	0
515 - Steel Protective Coating		72	sq. ft.	72	0	0	0
313 - Fixed Bearing		2	each	0	2	0	0
1000 - Corrosion		2		0	2	0	0
515 - Steel Protective Coating		18	sq. ft.	18	0	0	0
330 - Metal Bridge Railing		1584	ft.	1573	8	3	0
1010 - Cracking		2		0	2	0	0

National Bridge Elements Inspection type: Fracture Critical,Routine Inspection Date: 9/12/2018

Bridge No: 04180

inspected by:	Infrastructure Engineers		

1020 - Connection		3		0	0	3	0
7000 - Damage		6		0	6	0	0
331 - Reinforced Concrete Bridge Railing	Mod.	1584	ft.	1312	192	80	0
1080 - Delamination/Spall/Patched Area		94		0	14	80	0
1090 - Exposed Rebar		3		0	3	0	0
1130 - Cracking (RC and Other)		175		0	175	0	0

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

FRACTURE CRITICAL MEMBERS / FRACTURE PRONE DETAILS

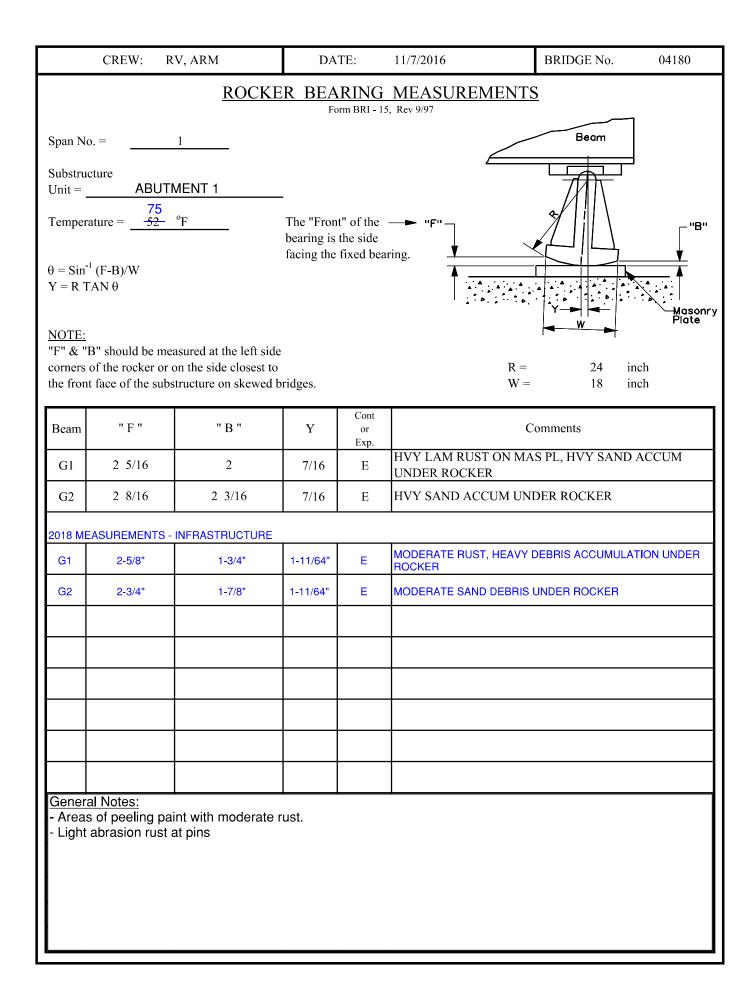
Inspectors:		Visits:			
Lead Inspector:	Matt Riesenberg	Visit Date:	Temp:	Start Time:	End Time:
Inspector:	Task:	09/12/2018	75	08:30 AM	03:30 PM
Area, 12	BSE - Manager	09/13/2018	70	08:00 AM	01:00 PM
Liguore, Matt	BSE - Inspector Rail - Inspector				
O'hara, Mike	BSE - Inspector				
Riesenberg, Matt	BSE - Inspector				
	Critical Inspection Frequency: 24 Monthe		irders		
Structure Type: Highway	v Bridges Year Built: 1977 ADT: 39	9795 Year o	f ADT: 2	018 % Tru e	ck: 9
Access Equipment Need	ded: 60' snooper and Catwalk.				
Traffic Control Required	I: Shoulder closure on I-84 Westbound.				
Reference to Plans:	Project No. 96-115 (1975)				
	· · · · ·				
	MEMBER/DET.	AIL TYPE # 1			
Member/Details Type:	A One or two steel girder systems			Fracture	Critical: Yes
Fatigue Category:	E Steel Type: A-588			Fatigue P	rone: Yes
Description:	Welded steel floorbeams and girders.				
200011011					
Inspection Procedure:	100% hands-on.				
Inspection Procedure: Condition Comments:					
	See BRI-18.	plain:			
Condition Comments:	See BRI-18.				
Condition Comments: Procedure Followed T	See BRI-18. his Inspection? Yes If No please ex	AIL TYPE # 2		Fracture	Critical:

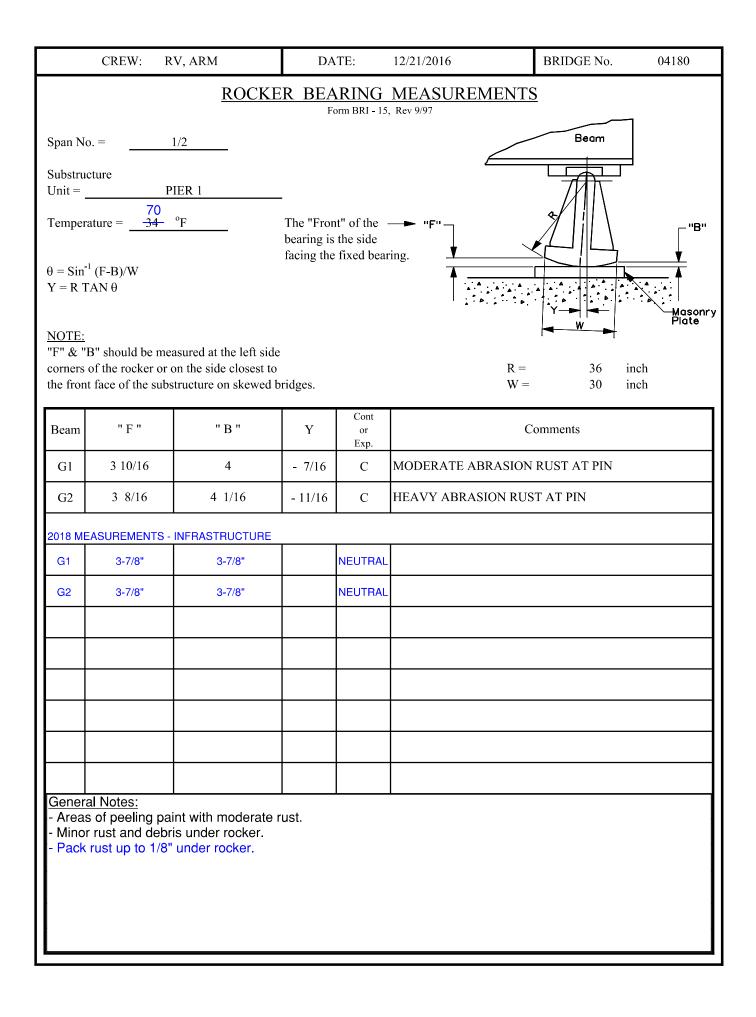
Form: BRI-12, Rev. 1/1 Inspection type: Fractor Inspection Date: 9/12/2 Inspected by: Infrastruc	ure Critical,Routine Bridge No: 04180	Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS
Description:	Horizontal lateral bracing gusset plate welded connec girder webs in tension zones.	ctions to floorbeam bottom flanges and lower
Inspection Procedure:	100% hands-on.	

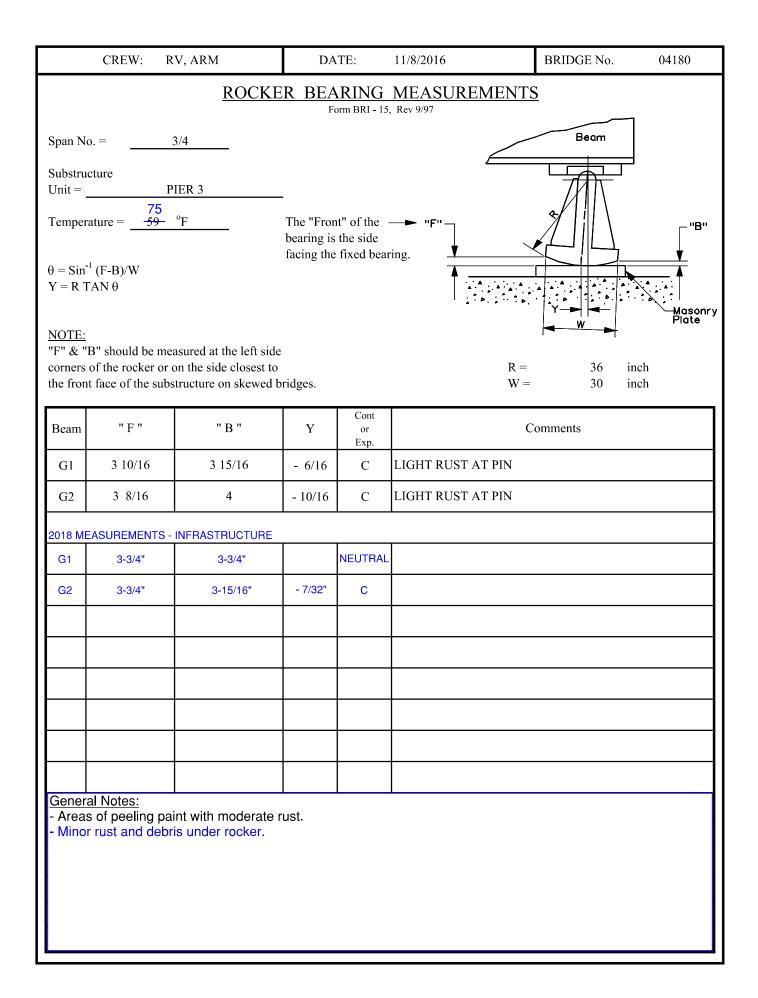
Procedure Followed This Inspection? Yes

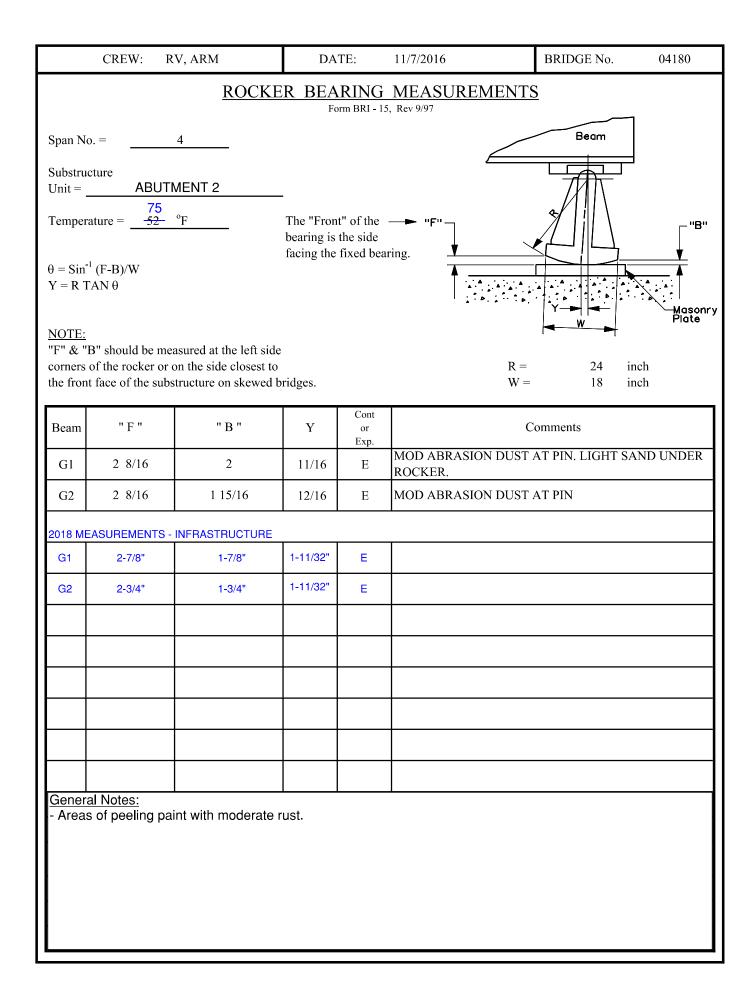
Condition Comments: No notable deficiencies.

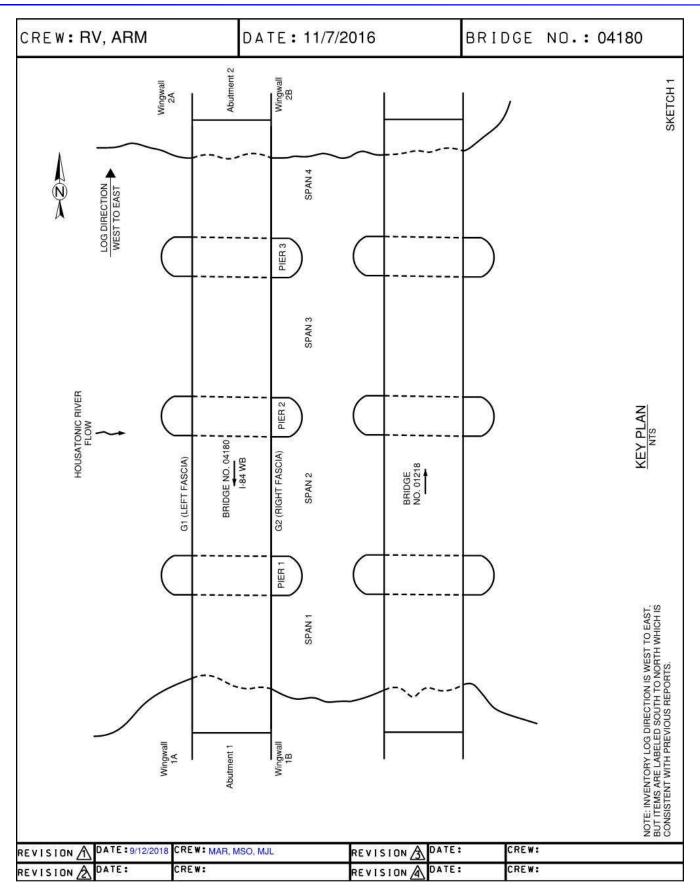
If No please explain:



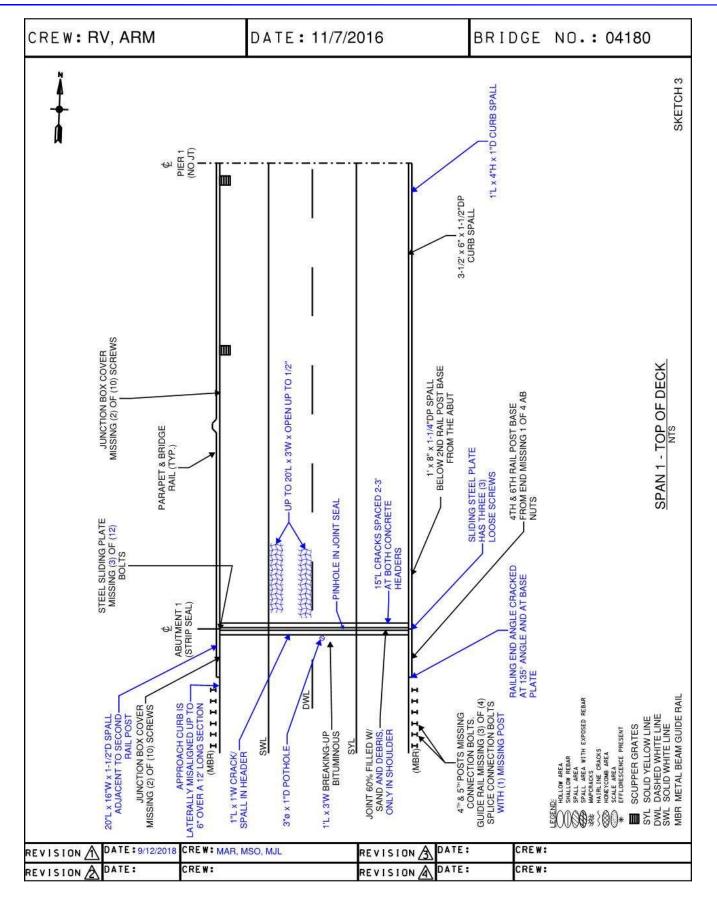


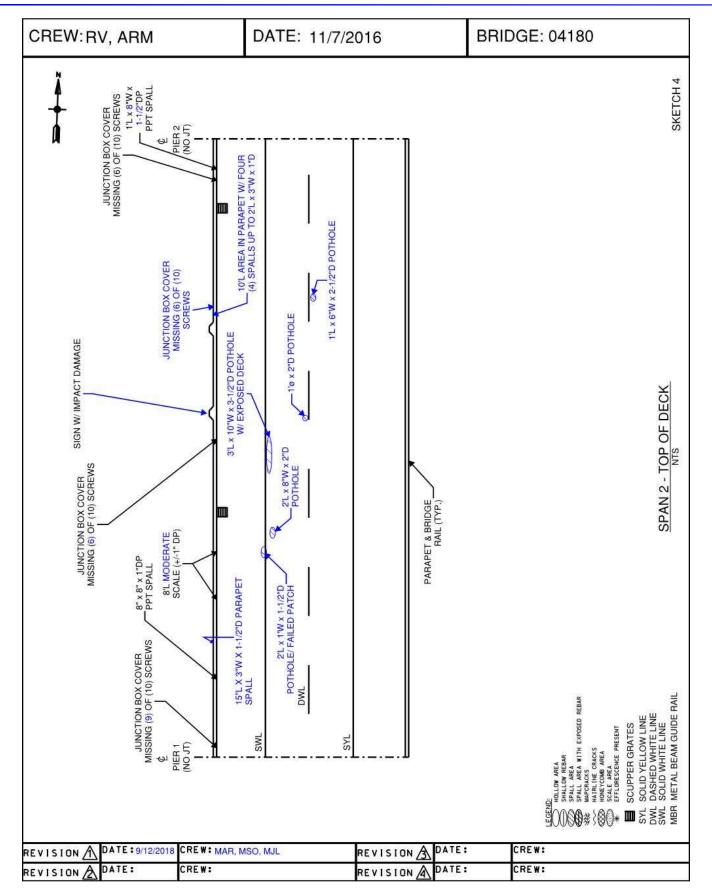


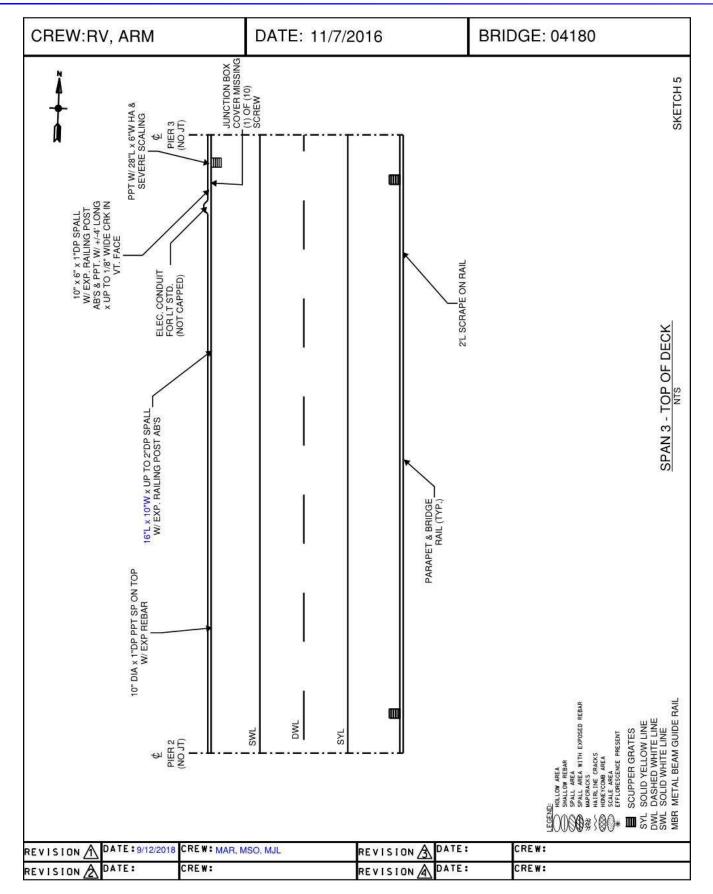


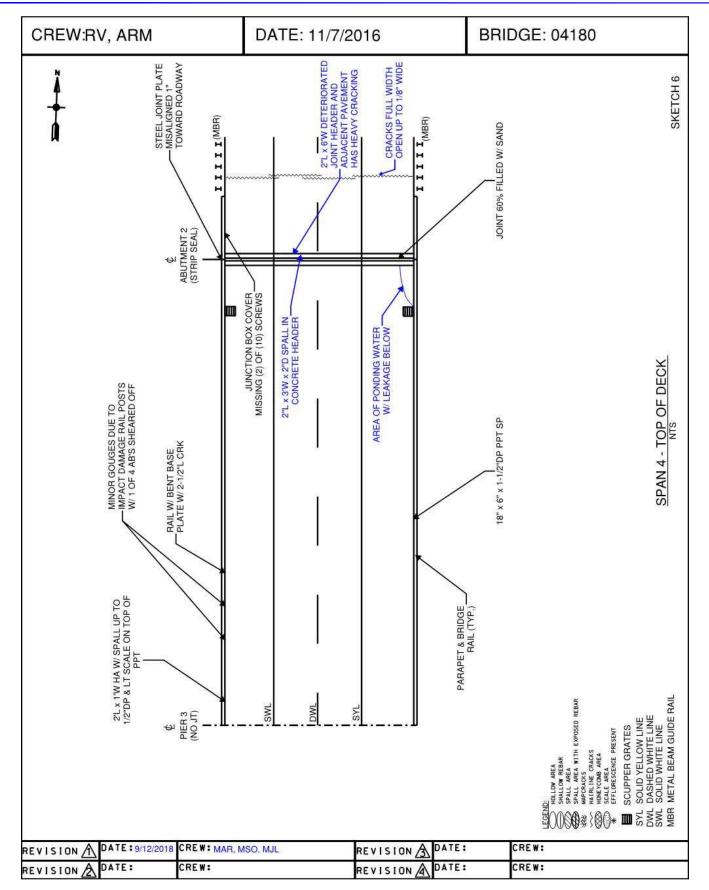


REW:RV,ARM	DATE: 11/7	//2016	BRIDGE	ND.: 04	18
		ECK GENERAL NO	TEQ		
		CA GENERAL NO	TES		
BITUMINOUS CONCRETE OVERLAY:					
LIGHT WEAR WITH MODERATE WEAR		NTS.			
RANDOM SHORT CRACKS OPEN UP T					
UP TO 1/4" DEEP MILLED AREAS ALON		-			
 BITUMINOUS PATCHES IN THE SHOUL PAVING SEAMS OPEN UP TO 3"W, WO 		E.			
SLOPED GRANITE CURBS:					
• MINOR SCRAPES WITH RUST STAINS.					
• RANDOM SPALLS UP TO 3-1/2' LONG x	6" HIGH x 1-1/2" DEEP.				
AVERAGE CURB REVEAL:					
LEFT (WEST) SIDE = 3-3/4"					
RIGHT (EAST) SIDE = 3-1/4"					
REINFORCED CONCRETE PARAPETS:					
RANDOM AREAS OF SCALING UP TO					
SPALLS UP TO 18" LONG x 10" WIDE x	2" DEEP WITH AND WITHOUT E	XPOSED REBAR. SOME SI	PALLS UNDERMINE TH	E RAILING POST BASE	ES AN
HAVE EXPOSED ANCHOR BOLTS.					
RANDOM HORIZONTAL AND FULL HEI		UP TO 1/8" WIDE.			
RANDOM HOLLOW AREAS UP TO 28" I	ONG X UP TO 1' WIDE.				
SINGLE PIPE ALUMINUM RAILING:					
RANDOM COLLISION SCRAPES.					
RANDOM LOOSE ANCHOR NUTS.					
DRAINS:					
SCUPPER GRATES ARE PARTIALLY CL	OGGED UP TO 50% WITH SAN	O AND VEGETATION, BUT F	PIPES ARE CLEAR.		
STRIP SEAL EXPANSION JOINTS:					
LIGHT TO HEAVY ACCUMULATION OF	SAND AND DEBRIS (60% OF TH	E LENGTH), MOSTLY IN TH	E SHOULDERS.		
CONCRETE HEADERS HAVE RANDOM	FULL LENGTH TRANSVERSE C	RACKS SPACED 2'-3'.			
ACTIVE JOINT LEAKAGE AT BOTH ABU	TMENTS.				
APPROACH GUIDE RAILS:					
MINOR SCRAPES AND DENTS.					
APPROACH PAVEMENT:					
• MINOR WEAR.					
PAVING SEAMS OPEN UP TO 1/8" WIDE	La				
				SKE	TCH
VISION A DATE: 9/12/2018 CREW	MAR, MSO, MJL	REVISION	TE: CREW:		

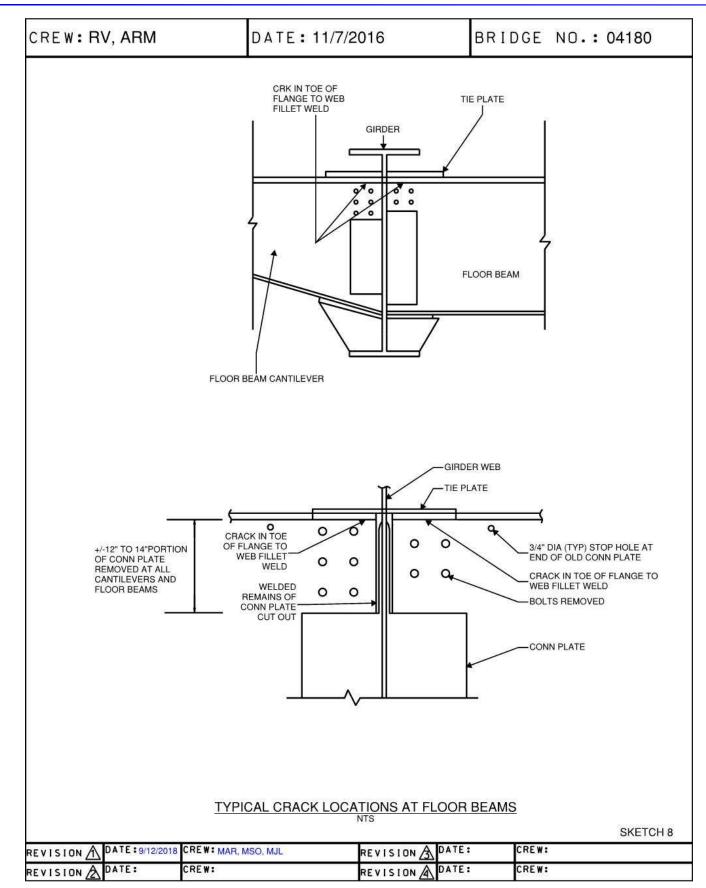


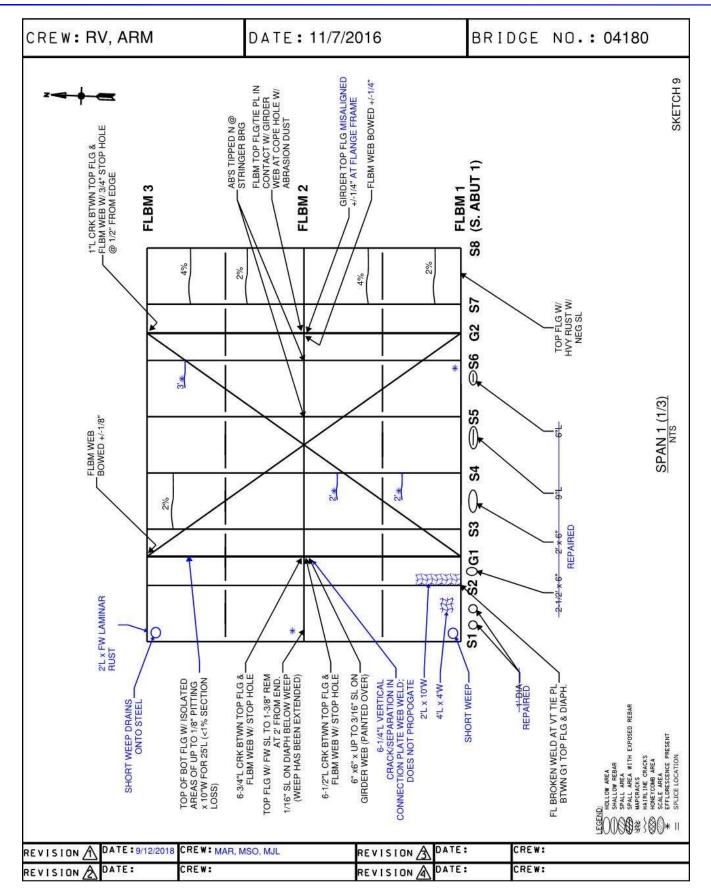


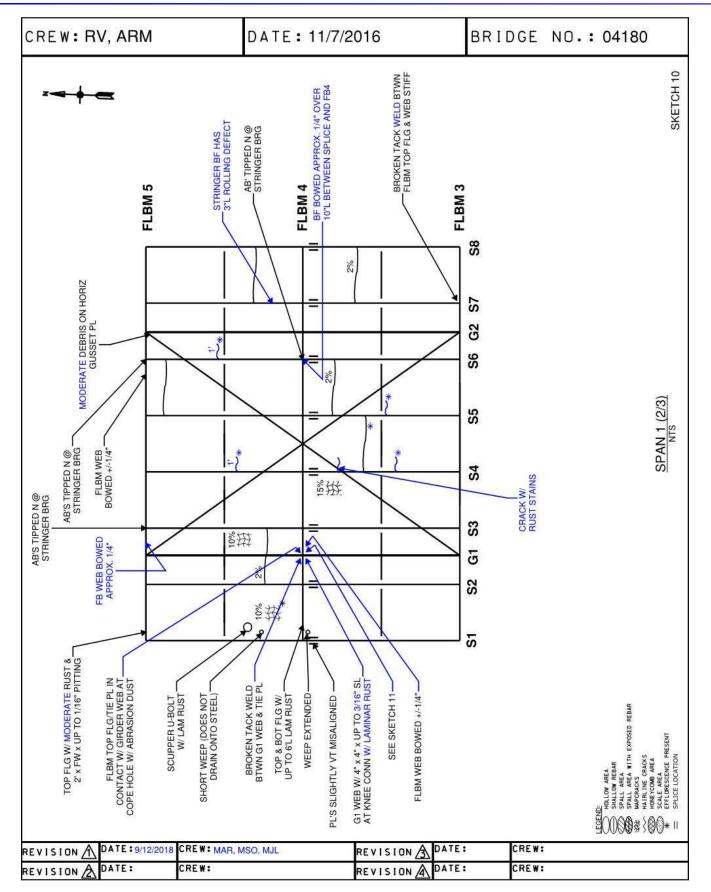


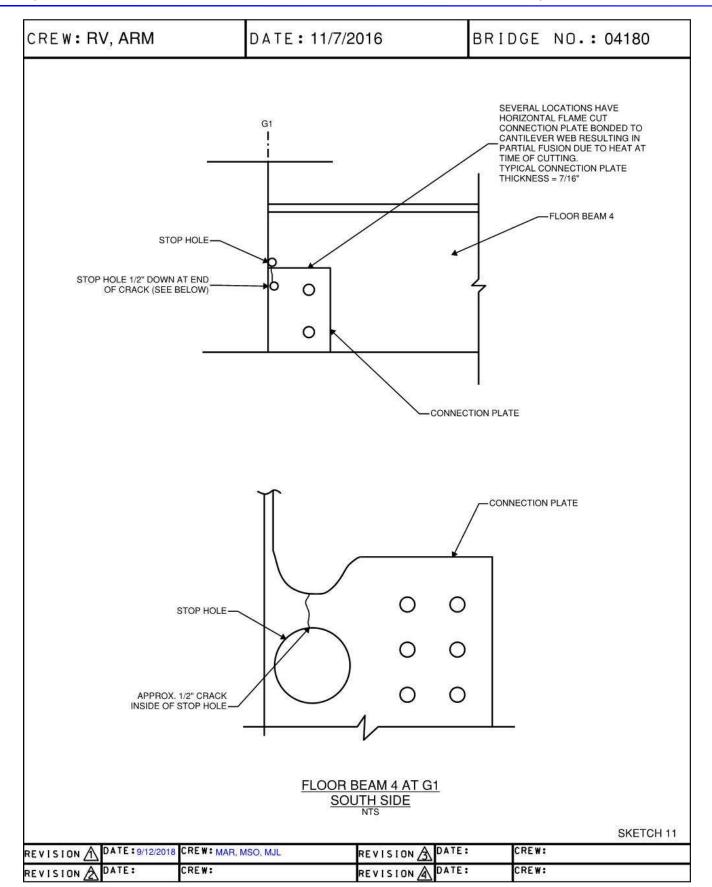


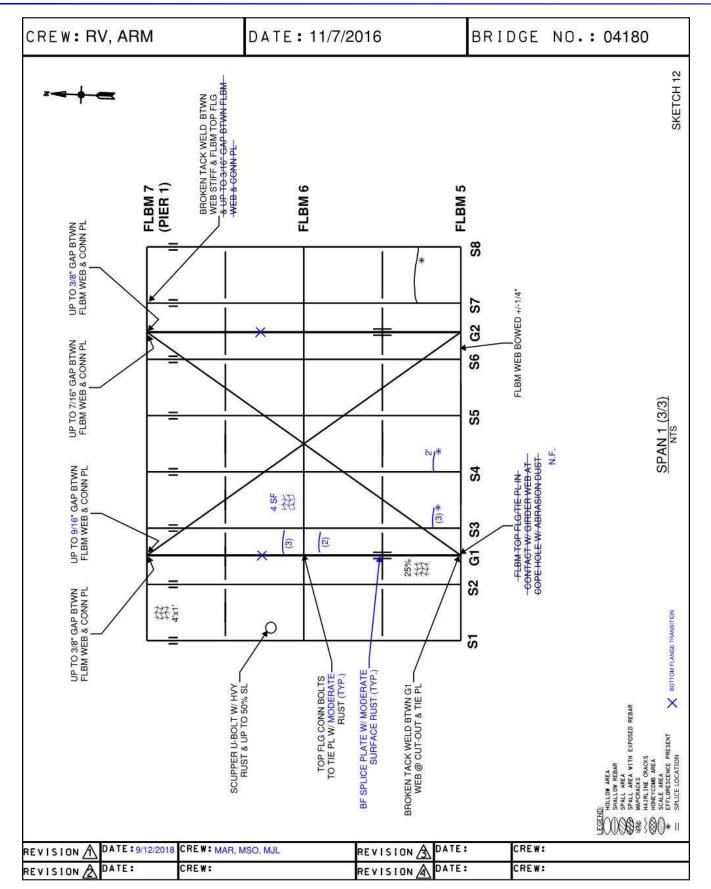
CREW:RV	, ARM	I	DATE: 11/7/2	016	BRID	DGE	NO.: 04180
		DECK	AND FRAMING F	PLAN - GENE	BAL NOTES		
		2201					
UNDERSIDE OF D							
RANDOM AREAS							OPERCENCE
· AREAS OF MOIS			AP CRACKS UP TO 100%	OF THE BAT ANE			LORESCENCE.
BEARING DEVICE	· G ·						
GIRDER ROCKE				-			
		EAVY ABRASION I	AHT TO MODERATE RUST				
			K BELOW ROCKERS.				
			SAND ACCUMULATION B	TWEEN MASONB		CKERS	IMPEDES NORMAL
MOVEMENT).						enerio (
and a second	TYPE BEARINGS	AT PIER 2 HAVE	AREAS OF PEELING PAIN	T AND LIGHT RUS	IT.		
STRINGER BLOG					D7-7		
		ND LIGHT TO MOD	ERATE RUST.				
- RANDOM BE	ARINGS W/SHIM	PLATES.					
- RANDOM GA	PS (UP TO 1/4") (GAPS BETWEN ED	GES OF BEARING/SHIM	PLATE AND FLOOP	BEAM TOP FLANC	GE W/ OF	R W/O PACK RUST.
STRINGERS:							
The second s	ING PAINT AND I	MODERATE RUST					
ISOLATED AREA	S W/ HEAVY RUS	ST W/ & W/O MINC	R SECTION LOSS UNLES	S OTHERWISE NO	TED.		
GIRDERS:							
			RATE RUST, MOSTLY AT	TOP FLANGES DU	E TO PAST WATEF	RLEAKAG	GE.
		N GIRDER WEBS.		-			
		ACE CONNECTION	S W/ RANDOM AREAS O	- SECTION LOSS (JP TO 6" HIGH x 6" 1	WIDE X 3	/16" DEEP RESULTING IN LESS
THAN 5% SECTIO	10001-0000-0000				ACE		
RANDOM BOWE			IDTH x 1/8" DEEP SECTIC	IN LOSS AT THE B	ASE.		
			TING UP TO 25' L X UP T			THAN 1	e/ CI
) TOP FLANGES +/- 1/4" (/				
FLOOR BEAMS:							
• FLOOR BEAMS A	AND CANTILEVER	RS HAVE RANDON	AREAS OF PEELING PA	INT WITH UP TO M	ODERATE RUST.		
RANDOM GIRDE	RS AND FLOOR	BEAMS HAVE BOV	VED/WARPED WEBS.				
							SKETCH 7
VISION A	ATE: 9/12/2018	CREW: MAR, MS	O, MJL	REVISION	DATE:	CREW:	MERICAN - 22 (2011)
VISIONAD	ATE:	CREW:		REVISION	DATE:	CREW:	

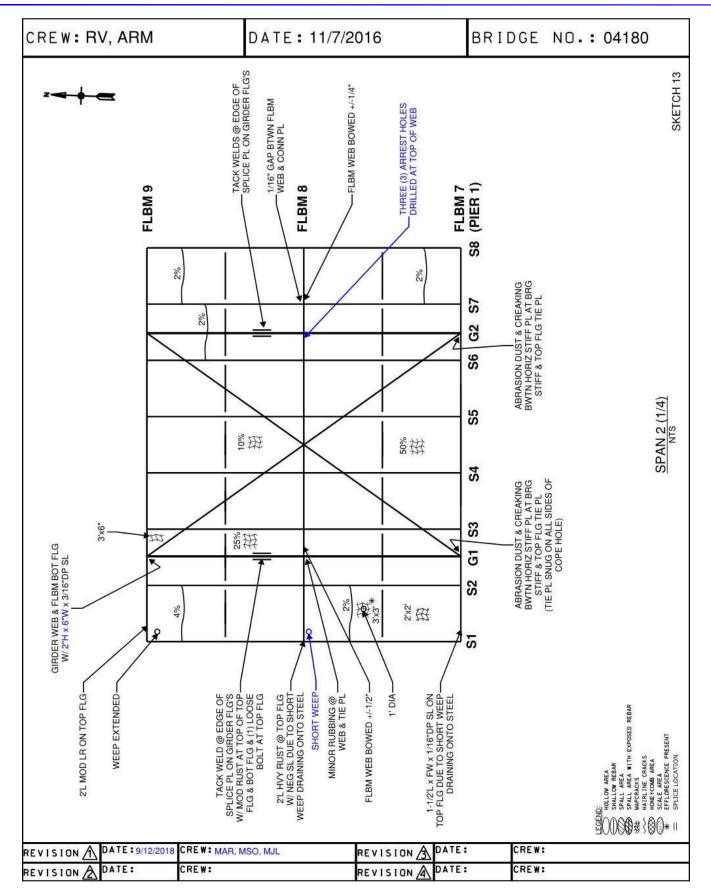


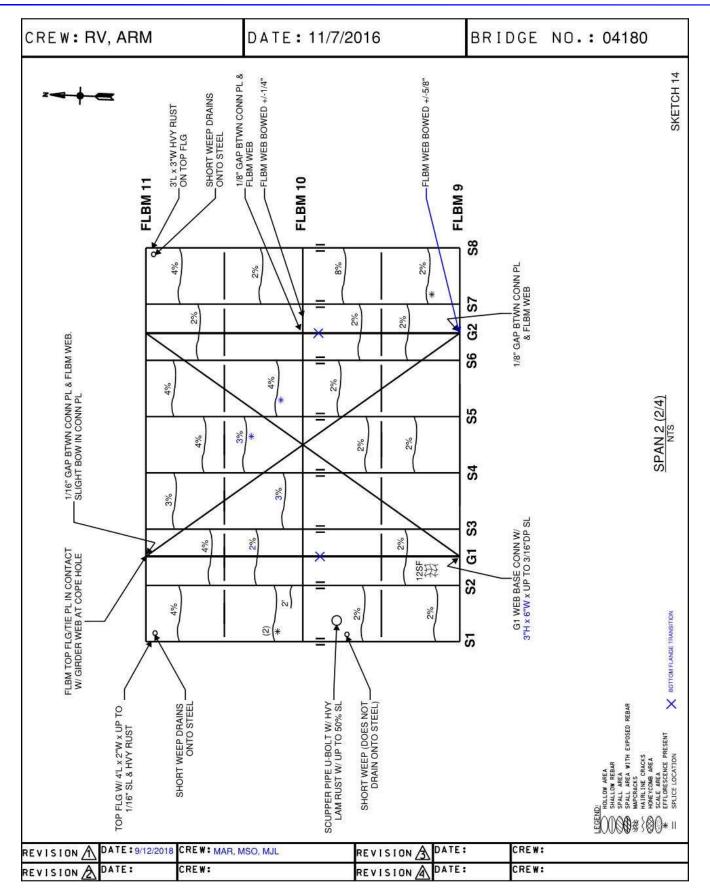


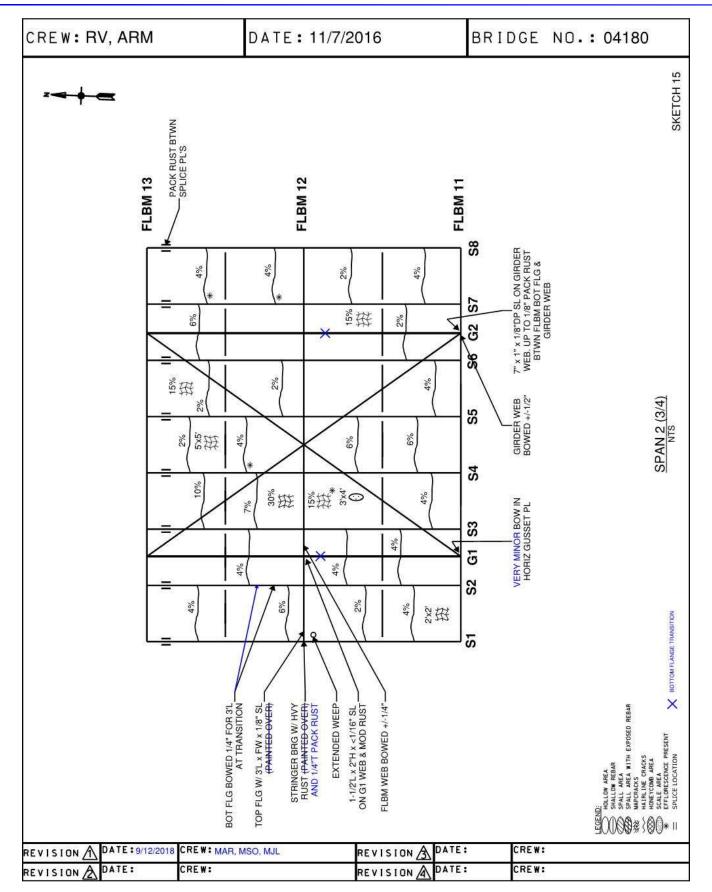


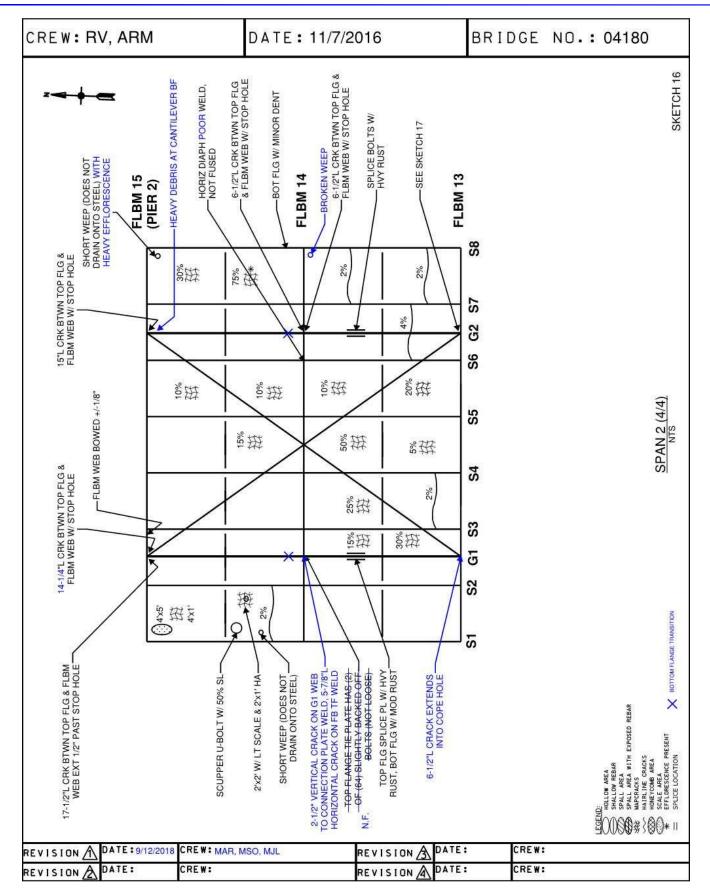




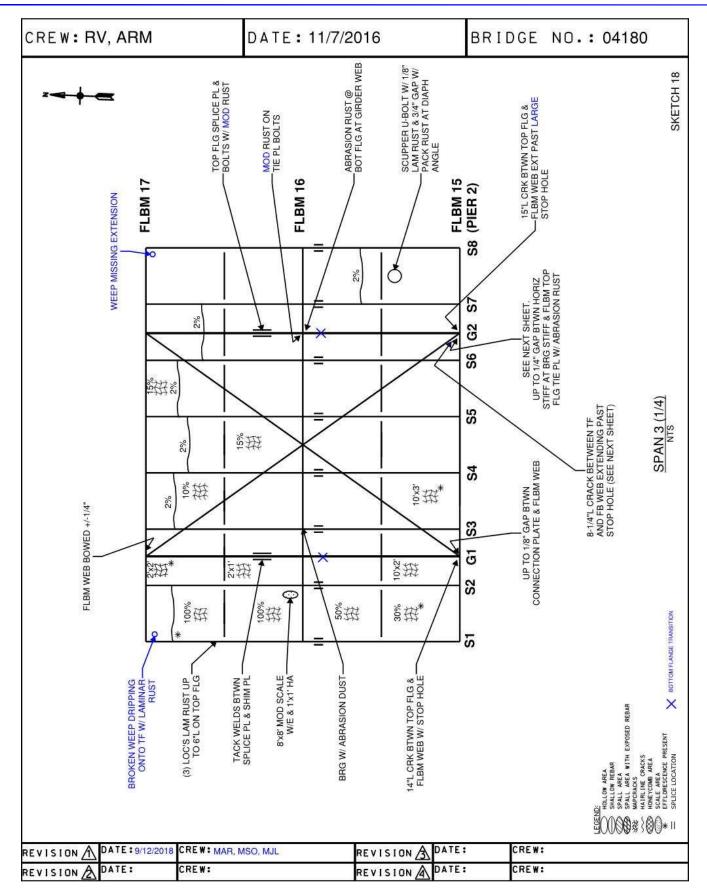


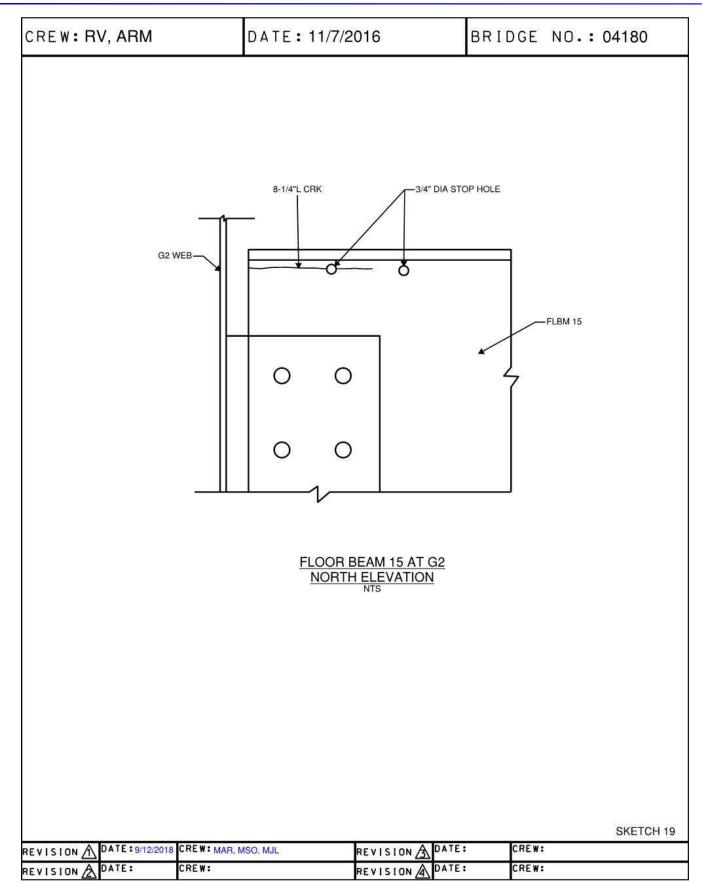


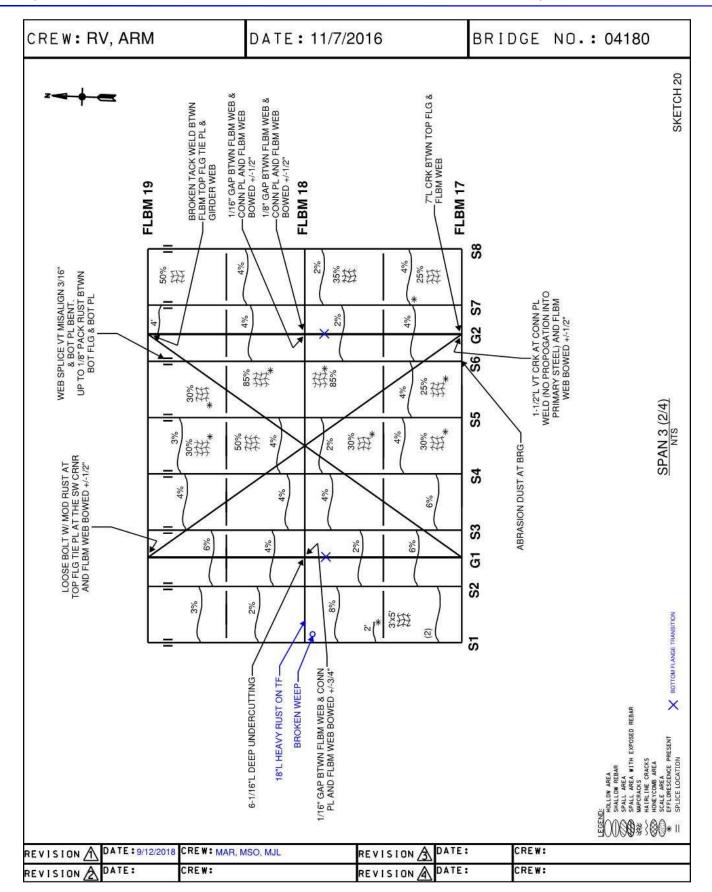


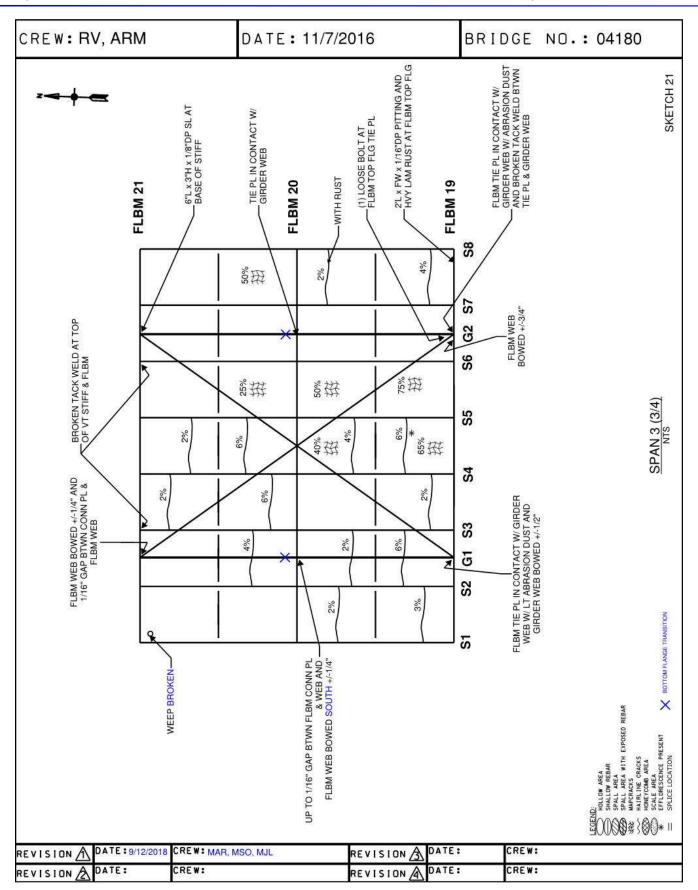


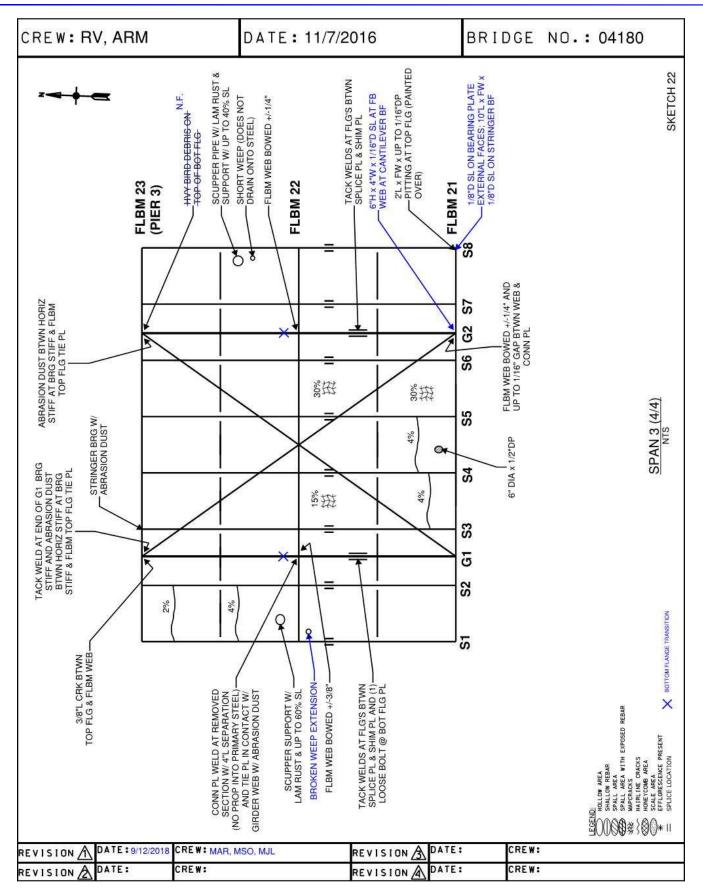
CREW:RV,ARM	DATE: 11/7/2016	BRIDGE	ND.: 04180
WELD (BOTH PREV CRACK	CRACK IN VERTICAL OF WEB OF FLBM 13 SIDES) WHICH WAS IOUSLY FLAME CUT. AT TOP OF WELD W/ PROPOGATION INTO PRIMARY STEEL	-CONNECTION PLATE	
REVISION A DATE: 9/12/2018 CRE	MI: MAR, MSO, MJL	A DATE: CREW:	SKETCH 1

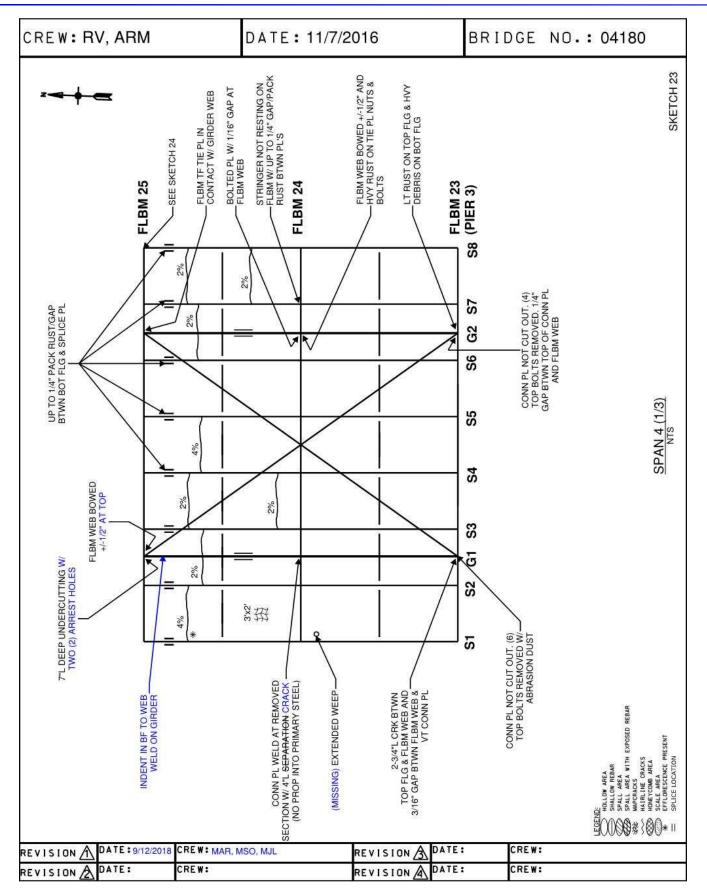




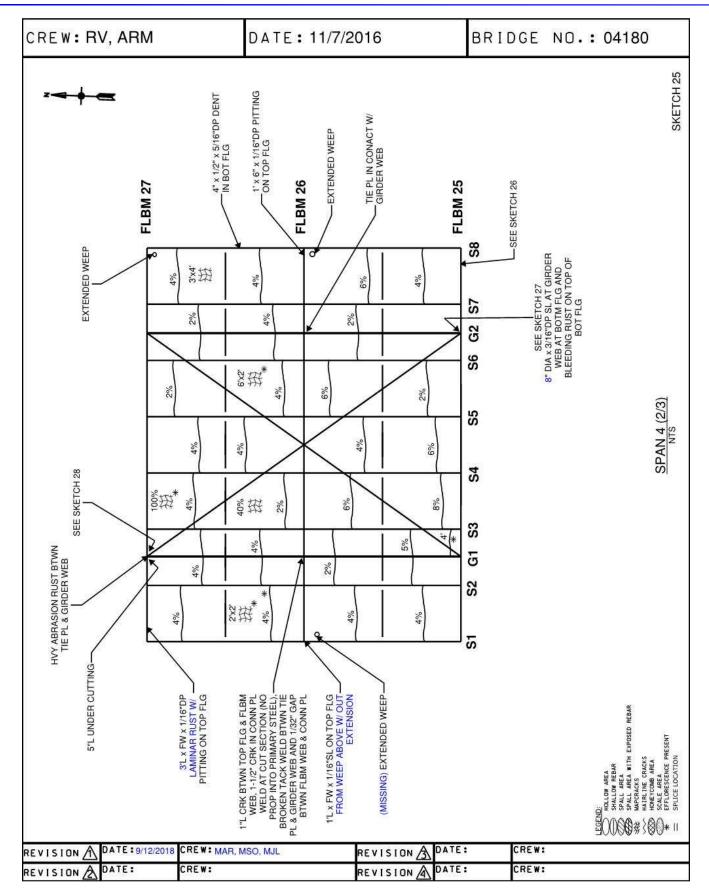


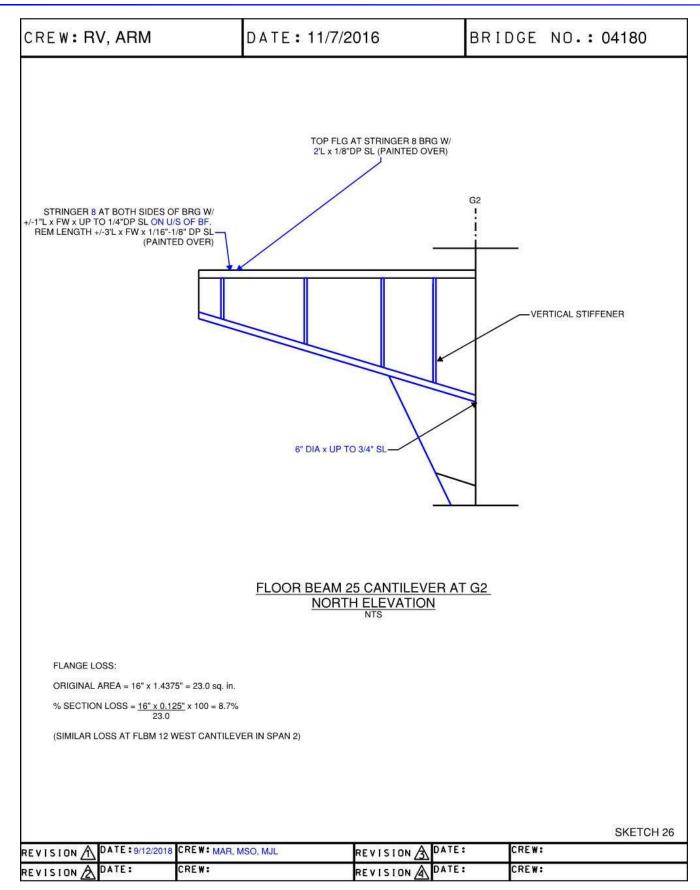


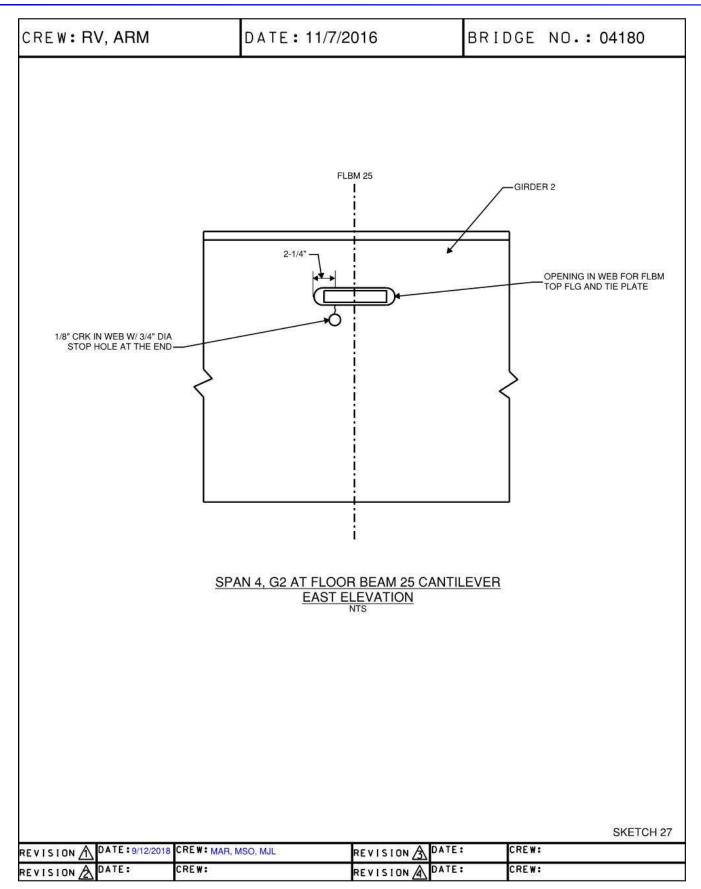


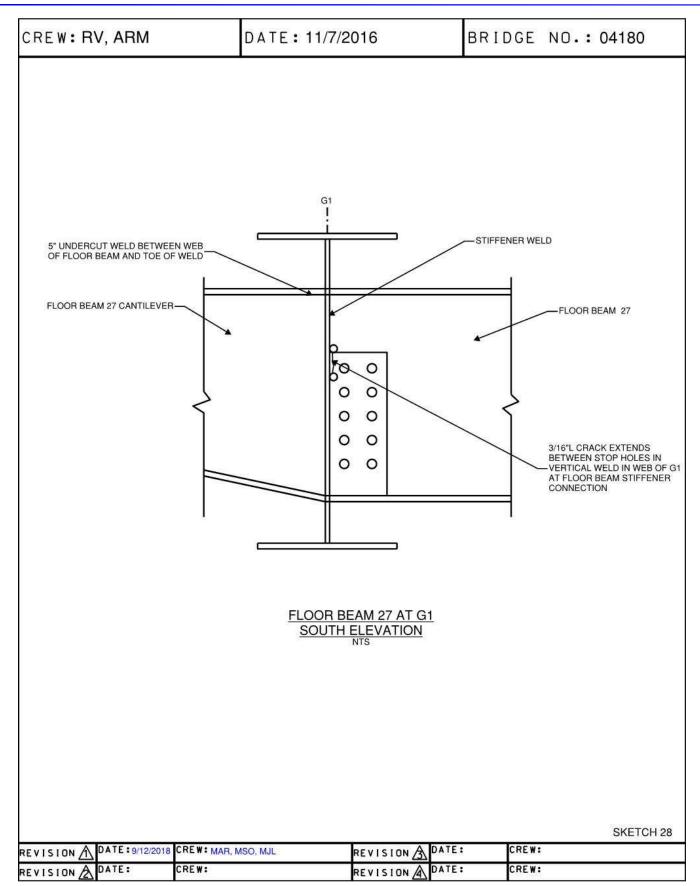


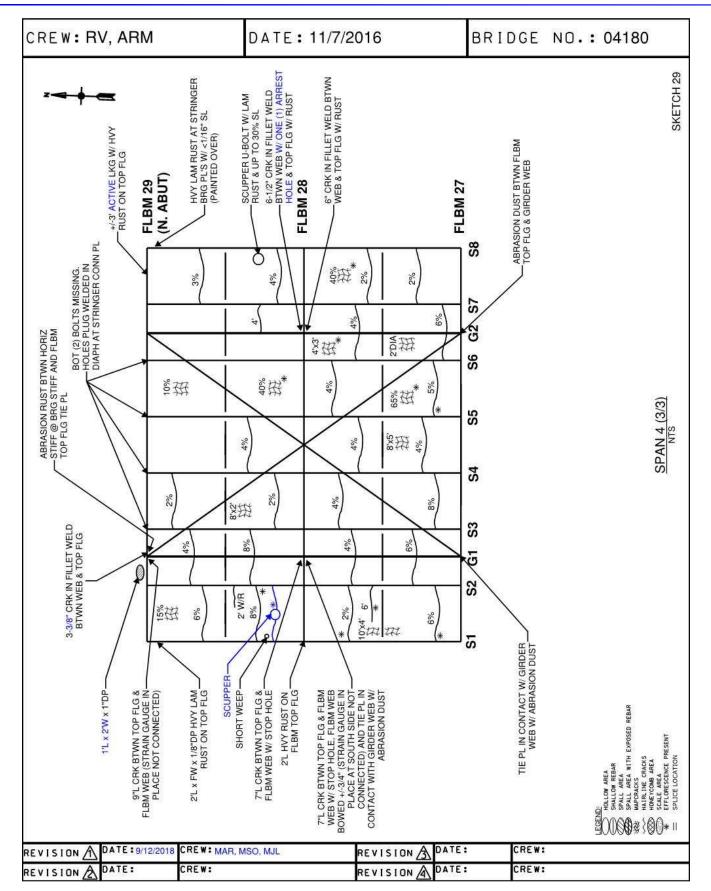
28	DATE: 11/7/2016	BRIDGE NO.: 04180
STRINGER SECTION: WF 24 x 84 OVERALL DEPTH, D = 24.1 in ; THICKNES DEPTH OF WEB, d _w = 22.56 in; THICKNES	SS OF FLANGE, $t_{\rm r}$ = 0.77 in ; WIDTH OF FLANGE, SS OF WEB, $t_{\rm w}$ = 0.47 in	$W_{\rm F} = 9.02$ in
=	FLBM 25	
4'L x 2"H x UP TO 3/16" DP SL (PAINTED OVER)		
STRINGER 8 AT BOTH SIDES OF BR +/-1"L x FW x UP TO 1/4"DP SL ON U/S OF REM LENGTH +/-3"L x FW x 1/16"-1/8" D (PAINTED O	BF. 1/4"D SL	3'L x 4"W SL DOWN TO 9/16" REM (PAINTED OVER) FLOOR BEAM 25
	WEST ELEVATI	
WEB LOSS FOR SHEAR: ORIGINAL AREA = 22.56" x 0.47" = % SECTION LOSS = <u>2" x 0.1875"</u> x 10.6	10.6 sq. in. % SECTION LC	OR BEARING (OVER BEARING): $OR = (0.47"5625") \times 100 = 39.9\%$ 0.47" OR BEARING (OVER BEARING):
FLANGE LOSS: ORIGINAL AREA = 9.02" x 0.77" = 6 % SECTION LOSS = <u>4" x 0.1875"</u> x <u>6.95</u>	6.75 sq. in.	DSS (AT MIN. REM) = <u>(3/16")</u> x 100 = 39.9% 0.47"
		SKETCH 2
		UNE TO TA









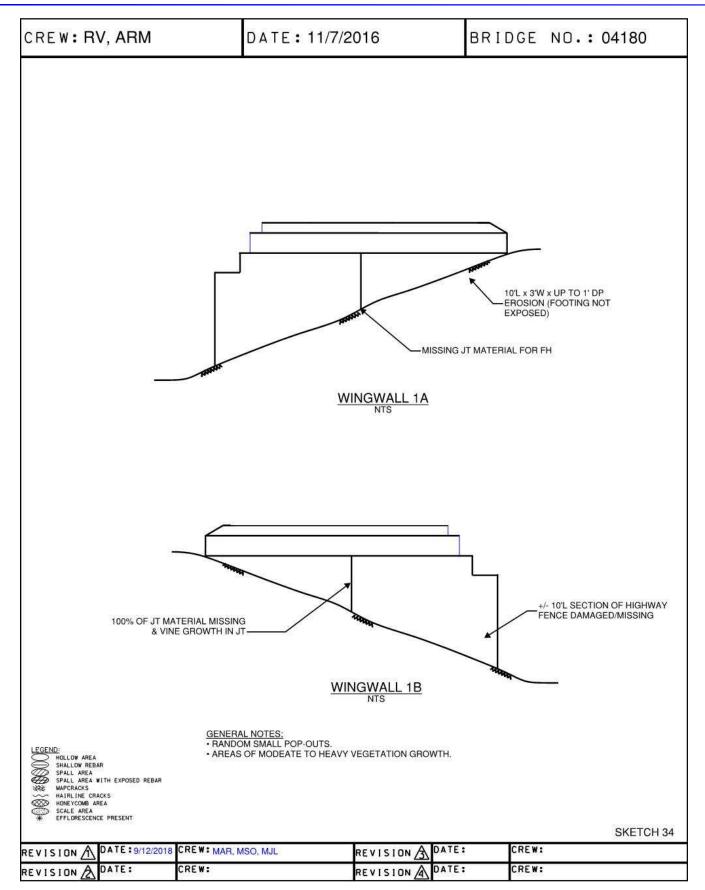


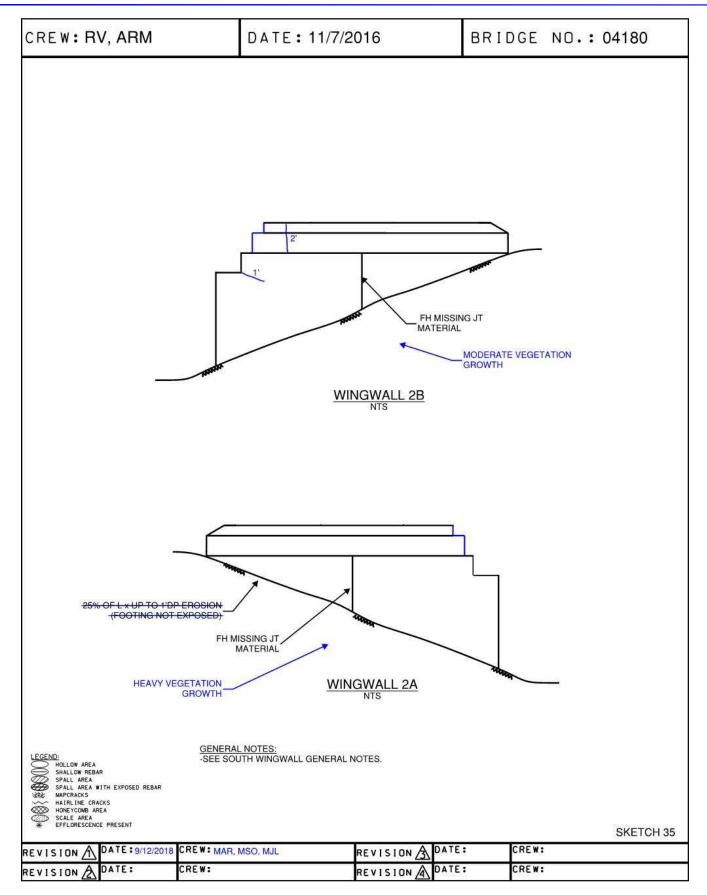
: RV, ARM	DATE: 1	1/7/2016	BRI	DGE NO.: 0418
		BEAM CRACK TAE		
ALL CRACKS BELOW HAVE A S	TOP HOLE IN PLACE UNLE	SS OTHERWISE NOTED.		
SEVERAL CRACKS HAVE BLEED	DING RUST OR ABRASION	RUST EMANATING FROM TH	E CRACK.	
SPAN 1:				
1. FLOOR BEAM 1, SOUTH ELE DIAPHRAGM VERTICAL TIE PLA		ONG CRACK IN THE WELD B	ETWEEN THE TO	P FLANGE OF G1 AND THE
2. FLOOR BEAM 2, NORTH ELE OF THE FILLET WELD BETWEEN		영상 이번 것은 것이 아파가 안에 가지 않는 것이 가지 않는 것이 같이 다.	n de la compacta de la construcción de la construcción Notes	HORIZONTAL CRACK IN THE TO
3. FLOOR BEAM 2, SOUTH ELE OF THE FILLET WELD BETWEEN		8		HORIZONTAL CRACK IN THE TO
4. FLOORBEA 3, NORTH ELEVA FLOOR BEAM AND THE WEB ST		R HAS A 1" LONG BROKEN T	ACK WELD BETW	EEN THE TOP FLANGE OF THE
5. FLOOR BEAM 3, SOUTH ELE BETWEEN THE TOP FLANGE AN			NTAL CRACK IN T	HE TOE OF THE FILLET WELD
6. FLOOR BEAM 4, SOUTH ELE BEAM-TO-GIRDER CONNECTION				
7. FLOOR BEAM 4, NORTH ELE FLOOR BEAM TOP FLANGE TIE		VER HAS A BROKEN TACK V	VELD BETWEEN	HE GIRDER WEB AND THE
8. FLOOR BEAM 5, NORTH ELE FLOOR BEAM TOP FLANGE TIE		VER HAS A BROKEN TACK V	ELD BETWEEN	THE GIRDER WEB AND THE
9. FLOOR BEAM 7, SOUTH ELE BEAM AND THE WEB STIFFENE		/ER HAS A BROKEN TACK W	ELD BETWEEN T	HE TOP FLANGE OF THE FLOOF
SPAN 2: 9A. FLOOR BEAM 13, SOUTH EL PLATE WELD.	EVATION AT WEST CANTIL	EVER HAS 2-3/4" VERTICAL	CRACK ON THE (GIRDER 2 WEB TO CONNECTION
10. FLOOR BEAM 14, SOUTH EL OF THE FILLET WELD BETWEEN			Aller o necessarian de la comuni	IORIZONTAL CRACK IN THE TOE
11. FLOOR BEAM 14, NORTH EL TOE OF THE FILLET WELD BET) HORIZONTAL CRACK IN THE
11A. FLOOR BEAM 14, SOUTH E WELD.	LEVATION AT WEST CANT	ILEVER 5-7/8" LONG HORIZO	NTAL CRACK ON	FLOOR BEAM TOP FLANGE
11B. FLOOR BEAM 14, NORTH E	ELEVATION AT WEST CANT	ILEVER HAS A 6-1/2" CRACK	THAT EXTENDS	INTO COPE HOLE.
12. FLOOR BEAM 15, SOUTH EL WELD BETWEEN THE TOP FLAM THE CRACK.				
13. FLOOR BEAM 15, SOUTH EL OF THE FILLET WELD BETWEE			e gepernen etabet terztettettettettettettettettettet. 1915	HORIZONTAL CRACK IN THE TO
ON A DATE: 9/12/2018 CRE	K: MAR, MSO, MJL	REVISION A	DATE:	CREW:
	W:	and the second	DATE:	CREW:

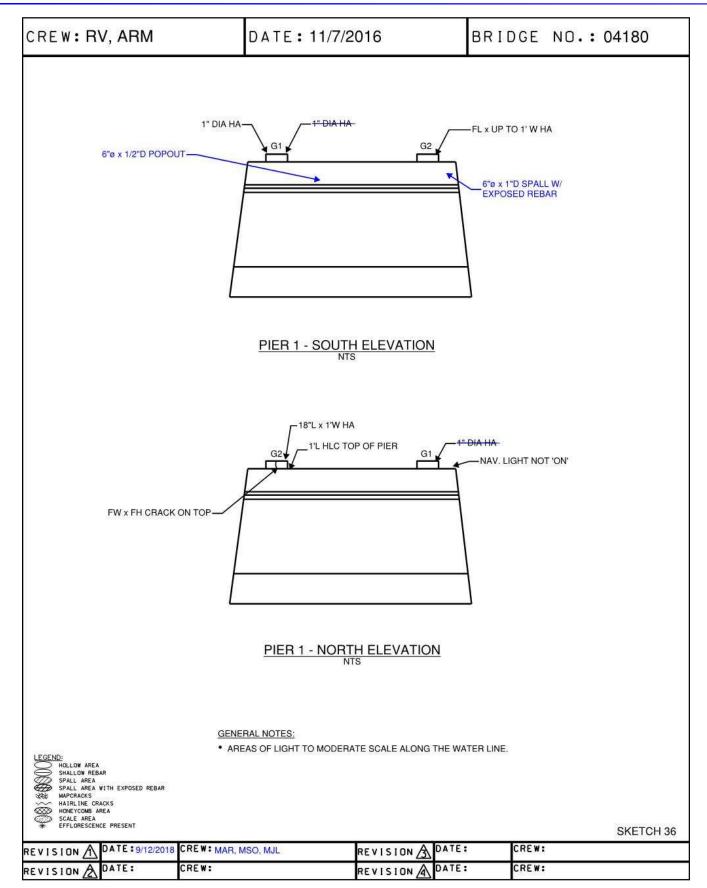
w:RV,ARM	DATE:	11/7/2016	BRI	DGE NO.:(0418
	FL OO	R BEAM CRACK TAE	RI F		
CDANO	1200				
SPAN 3: 14. FLOOR BEAM 15. SOUTH	ELEVATION AT WEST CANT	ILEVER HAS A 17-1/2" LONG F	IORIZONTAL CRA	CK IN THE TOE OF THE I	FILLET
WELD BETWEEN THE TOP F	LANGE AND THE WEB OF TH	E FLOOR BEAM (1/2" PAST ST	OP HOLE).		
		" (PREVIOUSLY 17-3/4") LONG OF THE FLOOR BEAM (1/2" P			IE
16. FLOOR BEAM 15, NORTH THE TOP FLANGE AND THE		1/4" LONG HORIZONTAL CRAC	CK IN THE TOE OF	F THE FILLET WELD BET	WEEN
		LEVER HAS A 15" LONG (PRE ND THE WEB OF THE FLOOR		HORIZONTAL CRACK IN	THE
	ELEVATIONS AT EAST CANTI LANGE AND THE WEB OF TH	LEVER HAVE A 7" LONG HORI E FLOOR BEAM.	ZONTAL CRACK I	N THE TOE OF THE FILL	ET.
19. FLOOR BEAM 19, SOUTH AND THE GIRDER WEB AT T		ROKEN TACK WELD BETWEEN	THE FLOOR BEA	AM TOP FLANGE TIE PLA	ATE
20. FLOOR BEAM 21, SOUTH THE WEB STIFFENER.	ELEVATION AT G1 HAS A BE	ROKEN TACK WELD BETWEEN	THE TOP FLANG	BE OF THE FLOOR BEAM	AND
20A. FLOOR BEAM 21, SOUT	H ELEVATION WEB STIFFENE	R TO TOP FLANGE AT STRIN	GER 6 HAS A BRC	DKEN TACK WELD.	
	ELEVATION AT WEST CANT	ILEVER HAS A 4" LONG VERT OPAGATION.	ICAL SEPARATION	N/CRACK AT THE TOP	
		ILEVER HAS A 3/8" LONG (PRI ND THE WEB OF THE FLOOR		HORIZONTAL CRACK IN	THE
SPAN 4:					
	HELEVATION AT WEST CANT LANGE AND THE WEB OF TH	ILEVER HAS A 2-3/4" LONG HO	ORIZONTAL CRAC	CK IN THE TOE OF THE F	ILLET
	ELEVATION AT WEST CANT	ILEVER HAS A 4" LONG VERT	ICAL CRACK AT T	HE TOP PORTION OF TH	ΗE
	ELEVATION AT EAST CANTI	LEVER HAS A 1/8" LONG VER	TICAL CRACK IN 1	THE G2 WEB FROM COP	E TO
	ELEVATIONS AT WEST CANT LANGE AND THE WEB OF TH	ILEVER HAVE A 1" LONG HOR E FLOOR BEAM.	IZONTAL CRACK	IN THE TOE OF THE FILL	LET
	HELEVATION AT WEST CANT NECTION PLATE WITH NO PR	ILEVER HAS A 1-1/2" LONG VE OPAGATION.	ERTICAL SEPARA	TION/CRACK AT THE TO	P
28. FLOOR BEAM 26, NORTH FLANGE TIE PLATE AND THE		ILEVER HAS A BROKEN TACK	WELD BETWEEN	THE FLOOR BEAM TOP	2
		ILEVER HAS A 3/16" LONG HO INNECTION PLATE (TWO STO			LLET
	ELEVATIONS AT WEST CANT LANGE AND THE WEB OF TH	ILEVER HAVE A 7" LONG HOR E FLOOR BEAM.	IZONTAL CRACK	IN THE TOE OF THE FILL	LET
	ELEVATION AT EAST CANTI	LEVER HAS A 6" LONG HORIZ E FLOOR BEAM.	ONTAL CRACK IN	THE TOE OF THE FILLE	т
	HELEVATION AT EAST CANTI LANGE AND THE WEB OF TH	LEVER HAS A 6-1/2" LONG HC E FLOOR BEAM.	RIZONTAL CRAC	K IN THE TOE OF THE FI	ILLET
33. FLOOR BEAM 29, SOUTH		ILEVER HAS A 9" LONG HORIZ	ZONTAL CRACK IN	N THE TOE OF THE FILLE	ET
34. FLOOR BEAM 29, NORTH THE TOP FLANGE AND THE		3/8" LONG HORIZONTAL CRA	CK IN THE TOE OF	F THE FILLET WELD BET	WEEN
<u></u>			······		SKE
ON A DATE: 9/12/2018 C	REW: MAR, MSO, MJL	REVISION	DATE:	CREW:	
	REW:	REVISION	DATE:	CREW:	

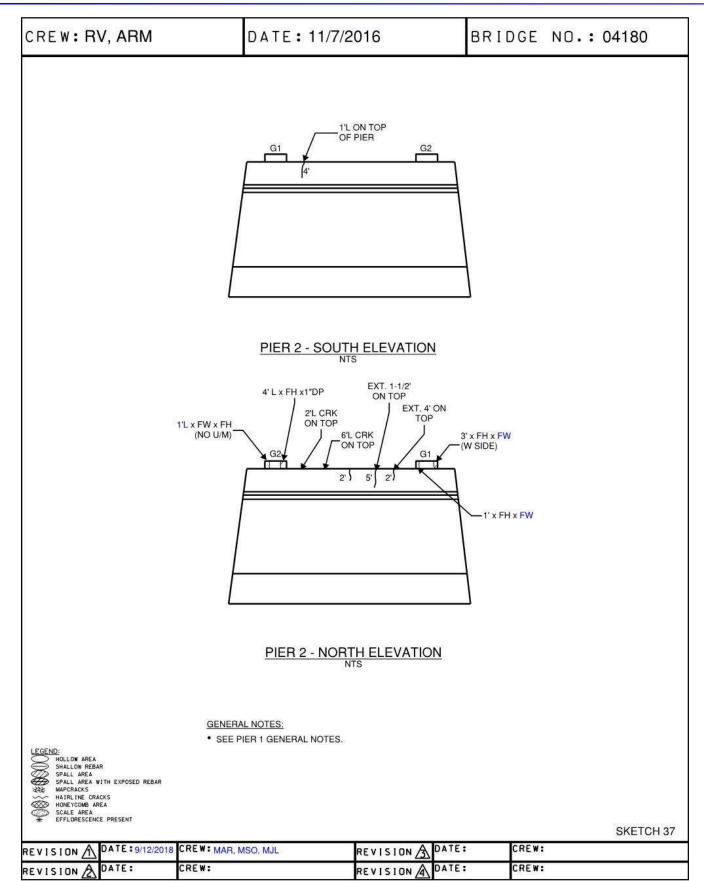
CREW:RV,ARM	DATE: 11/7/2016	BRIDGE	NO.: 04180	
SMALL HOLE IN JOINT 6" DIAMETER X 1/2" DP WIEXP REBAR 6" DIAMETER X 1/2" DP MEXP REBAR WIEXP REBAR	ACTIVE LEAKAGE	32" x 3" x 2-1/2" DP. SPALL AT N SIDE 30" x 3" X 2-1/2" DP. SPALL AT WSIDE 30" x 3" X 2-1/2" DP. SPALL AT WSIDE OF PEDESTAL	<u>ENERAL NOTES:</u> RANDOM AREAS OF LIGHT TO HEAVY GRAFFITI. EVIDENCE OF PAST ACTIVE DECK JOINT LEAKAGE WITH SILT STAINS .	ABUTMENT 1 NTS
EXISION DATE: 9/12/2018 CRE #: WAR, 1		DATE: CREW	G LEGEND: HOLLOW AFEA SHALL AFEA SPALL AFEA SPALL AFEA SPALL AFEA SPALL AFEA AFEAACEA MATERIA	SCALE AREA

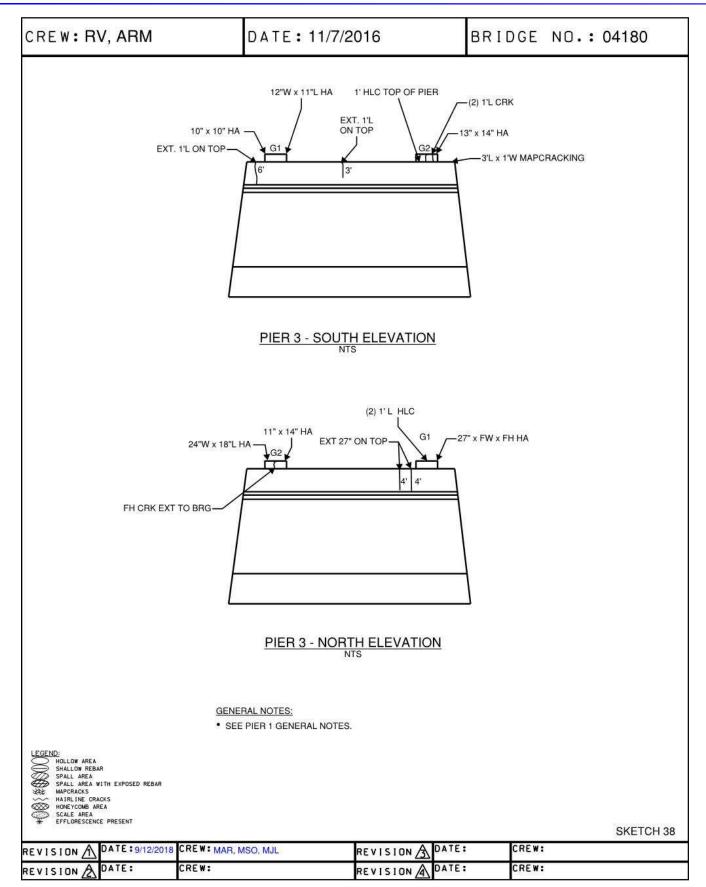
			5	and and		¥0@{&@@000E	
	2.5' x 8" x 2-1/2"DP EDGE OF CHEEK WALL CHIPPED OUT DURING CONSTRUCTION	1.* 6 [*] * 1/2 [*] BP	10" × 5" × 3/4"DP W/ ADJ. 1' × 10" HA	17"L x 18"H x UP TO 3-1/2"D CONC. CUT OUT/SPALL W/ TWO (2) EXPOSED REBARS		LEGEND: MOLTOW REEA SHALLOW REEAR SALL AREA SPALL AREA SPALL AREA MOLTORESCED REEAR MOLTORESCENCE DECAR MOLTORESCENCE PRESENT * EFLORESCENCE PRESENT	
	7 W/ EXP RUSTED REBAR		4/ 2/2 2/ G1	2' x 4" x 3/4"DP ON EAST FACE	GENERAL NOTES:	SEE SOUTH ABUTMENT (1) GENERAL NOTES.	AI
	2' x 10" x 1"DP W/ EXP REBAR		3			EHAL NOTES.	ABUTMENT 2 NTS
				4'L W/ RUST			
		-1" x 6" x 2"DP SP		AUST RUST			SKETCH 33
CREW:RV,ARM	l	DATE: 11/7/2	2016	BRI	DGE N	0.:04180	33











Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 1

Bridge Identification Number

Photo Taken: 09/12/2018



Photo Number: 2

Right Elevation

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 3

Left Elevation



Photo Number: 4

South Approach From Bridge

Photo Taken: 09/12/2018

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 5

Bridge From South Approach



Photo Number: 6

Bridge From North Approach

Photo Taken: 09/12/2018

Photo Taken: 09/12/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 7

Photo Taken: 09/12/2018

Bituminous Concrete Overlay in Span 2

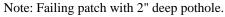




Photo Number: 8

Bituminous Concrete Overlay in Span 2

Photo Taken: 09/12/2018

Note: 3" deep pothole along paving seam with exposed concrete deck.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 9

Photo Taken: 09/12/2018

Bituminous Concrete Overlay in Span 2

Note: Failed patch along paving seam with potholes up to 2" deep.



Photo Number: 10

Span 4 Underside of Deck

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 11

Span 1 Underside of Deck

Note: Moisture.

Photo Number: 12

Photo Taken: 09/12/2018

Right Curb / Parapet

Note: Spall in the granite curb.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 13

Right Bridge Rail

Photo Taken: 09/12/2018



Photo Number: 14

Left Parapet in Span 2

Photo Taken: 09/12/2018

Note: 10' long section with (4) spalls up to 1" deep and scaling.



Photo Number: 15

Left Parapet



Note: Heavy scale on top face of parapet.

Photo Number: 16

Photo Taken: 09/12/2018

Southeast Approach Railing

Note: Two (2) cracks at end termination.



Photo Number: 17

Metal Bridge Rail Post

Note: Typical crack.



Photo Number: 18

Photo Taken: 09/12/2018

Span 4 Metal Bridge Rail

Note: Sheared anchor bolt.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 19

Deck Drain (Typical)

Note: Debris.



Photo Number: 20

Right Shoulder at Abutment 2

Photo Taken: 09/13/2018

Note: Ponding water.



Photo Number: 21

Scupper Pipe (Typical)

Photo Taken: 09/12/2018

Note: Laminar rust on inside of pipe, u-bolt and connection diaphragm.



Photo Number: 22

Photo Taken: 09/13/2018

Weep Drain Adjacent to Floor Beam 2 and Stringer 1 Note: Laminar rust with 1/16" SL on diaphragm below.



Photo Number: 23

Pier 2 Navigation Light (Typical)

Note: Not on during inspection.



Photo Number: 24

Abutment 1 Strip Seal Joint

Photo Taken: 09/12/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 25

Abutment 1 Header

Note: Crack and potential spall.



Photo Number: 26

Photo Taken: 09/12/2018

Left Parapet at Abutment 1

Note: Joint sliding plate missing 3 of 12 screws.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 27

Abutment 2 Joint

Photo Taken: 09/12/2018



Photo Number: 28

Left Parapet Sliding Metal Plate at Abutment 2

Note: Offset 1" from parapet.



Photo Number: 29

Northeast Approach Guide Rail

Photo Taken: 09/12/2018



Photo Number: 30

Southeast Approach Guide Rail

Photo Taken: 09/12/2018

Note: Disconnected post.



Photo Number: 31

Southeast Approach Guide Rail

Note: Missing splice bolt.



Photo Number: 32

North Approach Pavement

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 33

North Approach Pavement

Note: Cracking



Photo Number: 34

Girder 1 Rocker Bearing at Abutment 2

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 35

Girder 1 Rocker Bearing at Pier 1



Photo Number: 36

Girder 1 Bearing at Pier 3

Photo Taken: 09/12/2018



Photo Number: 37

Girder 1 Rocker Bearing at Abutment 1

Photo Taken: 09/11/2018



Photo Number: 38

Girder 2 Fixed Bearing at Pier 2

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 39

Stringer Bearing

1 noto 1 uken. 09/12/2010

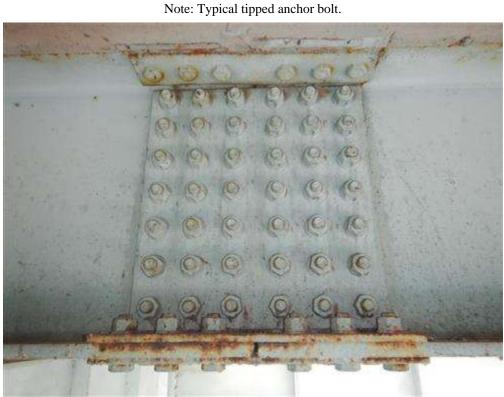


Photo Number: 40

Stringer Field Splice (Typical)

Photo Taken: 09/12/2018

Note: Up to 1/4" pack rust between bottom flange and bottom splice plate.



Bridge No: 04180

Photo Number: 41

Stringer 7 Bottom Flange Between FB4 FB



Photo Number: 42

Photo Taken: 09/12/2018

Stinger 8 Bottom of Bottom Flange at Floor Beam 25

Note: 3'L x 4" W section loss down to 9/16" remaining.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 43

Photo Taken: 09/12/2018

Stringer 8 Left Face at Floor Beam 25

Note: 4' long x up to 2" high x up to 3/16" deep pitting on web.



Photo Number: 44

Stringer 8 Between FB 26 and FB 27

Photo Taken: 09/12/2018

Note: $4" \log x 1/2"$ wide x 5/16" deep dent.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Stringer Connection to Floor Beam 29

Photo Number: 45

Photo Taken: 09/13/2018



Photo Number: 46

Girder Inside Face (Typical)



Photo Number: 47

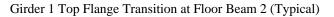
Girder Bottom Flange Transition (Typical)

Photo Taken: 09/12/2018



Photo Number: 48

Photo Taken: 09/12/2018



Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 49

Floor Beam (Typical)

Photo Number: 50

Floor Beam 25 Cantilever at Girder 2 (Typical)

Photo Taken: 09/12/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 51

Floor Beam 1 at Girder 1

Note: Full length broken weld at vertical tie plate between girder 1 top flange and diaphragm.



Photo Number: 52

Floor Beam 2 North Face at Girder 1

Photo Taken: 09/13/2018

Note: 6-3/4" crack between top flange and floor beam web.



Photo Number: 53

Floor Beam 2 at Girder 2

Note: Floor Beam web bowed 1/4".



Photo Number: 54

Photo Taken: 09/12/2018

Floor Beam 4 at Girder 1 (Inside Face)

Note: 1/2" long crack ends in stopper hole.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 55

Floor Beam 5 at Girder 1

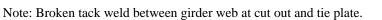




Photo Number: 56

Photo Taken: 09/12/2018

Floor Beam 7 Web at Girder 1

Note: 9/16" gap between connection plate at web.



Photo Number: 57

Floor Beam 7 Web at Girder 2

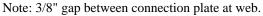




Photo Number: 58

Floor Beam 14 Cantilever at Girder 1

Photo Taken: 09/12/2018

Note: 2-1/2" vertical crack on connection plate weld and 5-7/8" horizontal crack on floor beam to top flange weld.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 59

Floor Beam 15 South Face at Girder 1

Note: 14-1/4" long crack between top flange and floor beam web with stop hole.



Photo Number: 60

Floor Beam 15 at Girder 2 North Face

Photo Taken: 09/12/2018

Note: 8-1/4" long crack between top flange and floor beam web.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 61

Photo Taken: 09/12/2018





Note: 15" long crack between top flange and floor beam web.

Photo Number: 62

Floor Beam 18 at Inside of Girder 1

Photo Taken: 09/12/2018

Note: 1/16" gap between floor beam web and connection plate and floor beam web is bowed 3/4".

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 63

Photo Taken: 09/12/2018

Floor Beam 19 Top Flange Tie Plate at Girder 1

Note: Loose bolt with moderate rust and floor beam web is bowed 1/2".



Photo Number: 64

Floor Beam 22 Left Cantilever at Girder 1

Photo Taken: 09/12/2018

Note: Connection plate weld at removed section has a 4" long vertical separation / crack that does not propagate into web.



Photo Number: 65

Floor Beam 25 at Girder 1

Note: Web bowed 1/2" at top.



Photo Number: 66

Floor Beam 25 Cantilever Bottom Flange at Girder 2

Photo Taken: 09/12/2018

Note: Girder 2 has 8" diameter x 3/16" deep section loss.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 67

Floor Beam 27 Left Overhang

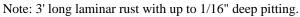




Photo Number: 68

Floor Beam 28 Cantilever at Girder 1

Photo Taken: 09/13/2018

Note: 7" long crack between top flange and floor beam web with stop hole.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 69

Floor Beam 28 Cantilever at Girder 2

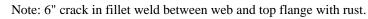




Photo Number: 70

Floor Beam 29 North Face at Girder 1

Photo Taken: 09/13/2018

Note: 3-3/8" crack in fillet weld between web and top flange.

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 71

Abutment 1 Elevation



Photo Number: 72

Girder 1 Pedestal at Abutment 1

Photo Taken: 09/12/2018

Note: Spall.



Photo Number: 73

Abutment 1 Right Cheekwall

Note: Spall.



Photo Number: 74

Abutment 2

Photo Taken: 09/12/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 75

Abutment 2 Near Girder 1 Pedestal





Photo Number: 76

Wingwall 2A Elevation

Photo Taken: 09/13/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 77

Pier 1 North Face

<image><image>

Photo Number: 78

Pier 3 North Face

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 WESTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 79

Channel Looking Upstream



Photo Number: 80

Channel Looking Downstream

Photo Taken: 09/12/2018