

BRIDGE NO.01218

52980 - NEWTOWN INTERSTATE 84 EASTBOUND over HOUSATONIC RIVER

Fracture Critical and Routine Inspection 9/10/2018 Inspected by: Infrastructure



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Report Title Page

Project No.: 170-3413 (Routine Inspections – NHS) Structure: 01218, Interstate 84 Eastbound over Housatonic River, Newtown Infrastructure Engineers, Inc. Inspection Date: 9/10/2018 Inspected By:

Professional Certification:

I hereby certify that this report, including all of its contents, has been approved by me, and that I am a duly licensed professional engineer under the laws of the State of Connecticut.



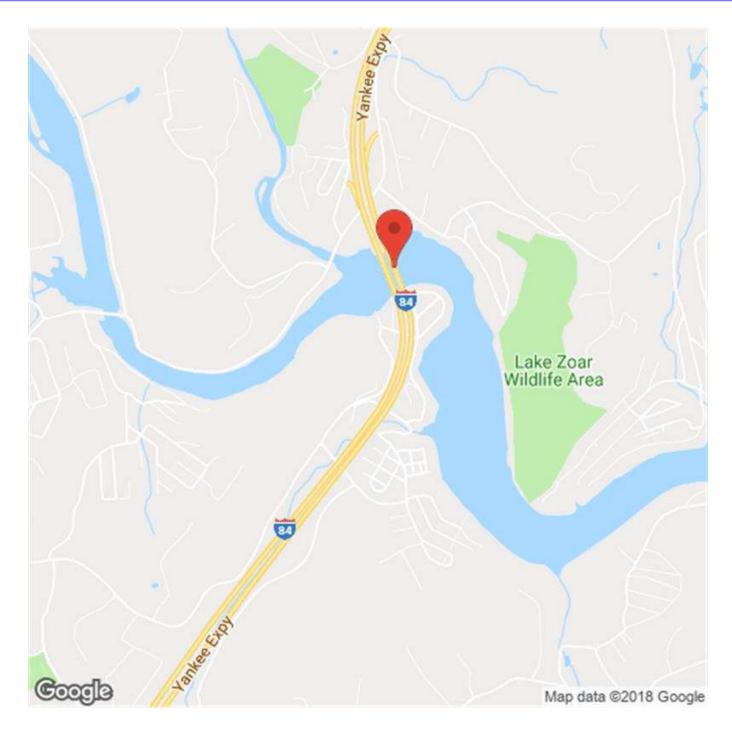
Jay Messier, PE (Infrastructure Engineers, Inc.)

PM / QAQC: Jay Messier, PE

CT License No.: 19023

Bridge No: 01218

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Location Map # 1

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

STRUCTURE INVENTORY & APPRAISAL

INSPECTION	STRUCTURE TYPE & MATERIALS
Structurally Deficient Y Functionally Obsolete N	(43) Structure Type, Main
Sufficiency Rating 62.4	A) Material 4 - Steel continuous
(90) Inspection Date 09/10/2018 (91) Frequency 24	B) Design Type 03 - Girder and Floorbeam System
Indepth Insp No Proposed next Indepth Year	(44) Structure Type, Approach
Deck Survey Date Class 03	A) Material 0 - Other
Access 11 - Less than 40 ft.reach 6	B) Design Type 00 - Other
Frequency Date Type	(45) Number of Spans, Main Unit 4
Fracture 24 09/10/2018 D Two Girder System, riveted / bolted plate girders	(46) Number of Approach Spans 0
Underwater 24 B Underwater Only	(107) Deck Structure Type 1 - Concrete Cast-in-Place
Special	(108) Wearing Surface/Protection Systems
	A) Type of Wearing Surface 6 - Bituminous
Bridge Name ROCHAMBEAU BRIDGE	B) Type of Membrane 2 - Preformed Fabric
Town Code - Name 52980 - NEWTOWN	
(5) Inventory Route	C) Type of Deck Protection 0 - None
(A) Record Type 1: Route carried "on" the structure	Substructure
(B) Signing Prefix 1 - INTERSTATE HIGHWAY	A) Material 2 - CONCRETE
(C) Level of Service 1 - MAINLINE	B) Design Type 1 - FULL HEIGHT STEM
(D) Route Number. 00084	Paint
(E) Dir Suffix 2 - EAST	Type 2 - Overcoated Lead Paint
(6A) Featured Intersected HOUSATONIC RIVER	Year 1978
(6B) Critical Facility Indicator	Comment
(7) Facility Carried INTERSTATE 84 EASTBOUND	GEOMETRIC DATA
(9) Location OVER HOUSATONIC RIVER	(48) Length of Maximum Span 224 ft.
(11) Mile Post 18.42 Miles	(49) Structure Length 792 ft.
(16) Latitude 41 Deg. 26 Min. 21 Sec.	(50) Curb or Sidewalk Widths
(17) Longitude -73 Deg. 14 Min. 51 Sec.	A) Left 0 ft. 0 in. B) Right 5 ft. 0 in.
(98) Border Bridge	(51) Bridge Roadway Width Curb to Curb 60 ft. 0 in.
(A) State Code (B) Percent Responsibility %	(52) Deck Width, Out to Out 70 ft. 1 in.
(C) Border Town Name	(32) Approach Roadway Width 60 ft.
(99) Border Bridge Structure No.	(,,,,,,,,,,

Form: BRI-19, Rev. 2/15 Inspection type: Fracture Critical,Routine Inspection Date: 9/10/2018 Inspected by: Infrastructure Engineers

(33) Bridge Median	0 - No median	A	AGE AND SERVICE
Deck Area 55519	sq. ft.	Year Built 1953	(106) Year Reconstructed 1978
 (34) Skew Angle (35) Structure Flared (35) Structure Flared (10) Inv. Rte. Min. Vert. Clearance (47) Inv. Rte. Total Horiz. Clr. Log Inv. Rte. Total Horiz. Clr. RLog Inv. Rte. Total Horiz. Clr. (53) Min. Vert. Clearence Over Bridg (54) Log-Min. Vert. Underclearance 	deg. lare 99 ft. 99 in. 60 ft. 0 in. 60 ft. 0 in. 0 ft. 0 in. 99 ft. 99 in.	(42) Type of Service	ay-pedestrian way B) Under 00
(55) Min. Lat Underclearance on Rig	ht N ref. 0 ft. 0 in.	(30) Years of ADT	2018
(56) Min. Lat Underclearance on Lef	t 0 ft. 0 in.	(19) Bypass, Detour Leng	th 1 Miles
CONDIT	ION		APPRAISALS
(58) Deck	5	(67) Structural Evaluation	4
(59) Superstructure	4	(68) Deck Geometry	9
(60) Substructure	6	(69) Underclearances, Ver	rt. & Horiz. N
(61) Channel & Channel Protections	5	(71) Waterway Adequacy	9
(62) Culverts	Ν	(72) Approach Roadway A	Nignment 8
(36) Traffic Safety Features		(113) Scour Critical	8
A) Bridge Railings	0		COMMENTS
B) Transitions	0	to "6". Refer to 2010 Ur • 07/28/2010; New Sec	Substructure) rating changed from "5" nderwater Inspection. RKD. ction Loss information submitted by Load Evaluation Group. RAP.
C) Approach Guardrail	0		
D) Approach Guardrail Er			
WATE			
Drainage Basin Waterway	6000 - Housatonic River	(112) NBIS Bridge Length	Yes
(38) Navigation Control	0 - No navigation control on waterway (bridge permit not required)	(104) Highway System	1 - Structure/Route is on NHS
(39) Navigation Vertical Clearance	0 ft.	(26) Functional Class	01 - Rural - Principal Arterial - Interstate
(40) Navigation Horiz. Clr.	0 ft.	(100) Defense Highway	1 - Is on an Interstate STRAHNET route
(111) Pier/Abutment Navigation	2 - In place and functioning	(101) Parallel Structure	R - Right structure (North or East)
(116) Vert-Lift Brg Nav Min	0 ft. 0 In.	(102) Direction of Traffic	1 - 1-way traffic

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND **Crossed:** HOUSATONIC RIVER Inventory Route: NHS

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(103) Temporary Stru	cture						
(110) Designated National 1 - Inver Network Network		entory route on National Truck k					
(20) Toll		3 - On Free Road					
(21) Maintain		01 - Sta	ate High	nway	Agenc	y	
(22) Owner		01 - Sta	ate Higł	nway	Agenc	y	
Report Class		S - STA	TE				
(37) Historical Signific	ance	5 - Not	eligible	for N	lational	Register	
	— РС	STED	SIGN	s –			
Other Posted Sign 1							
Other Posted Sign 2							
			Actual		Reco	mended	
Posted Load Single L	Jnit Tru	ck					tons
Posted Load Semi-Tr	ailer Tr	uck					tons
Posted Load 4 Axle T	ruck						tons
Posted Load 3S2 Tru	ck						tons
All Vehicles							tons
Posted Vert. Clearand	ce on E	ridge		ft.]in.	
Posted Vert. Underclearance		e		ft.]in.	
Posted Speed Limit of	on Bridg	je]m.p	.h.		
	- отн	ER FE/	ATURE	ES -			
Fence Required	No						
Fence Present	Yes						
Fence Type	1 - Ex	panded	Metal				
Fence Height	7.3						
Fence Material	1 - Alu	iminium					
Fence Top Type	Fence Top Type 2 - Return						
Barrel Ladders No							
Stand Pipes	Stand Pipes No						
Catwalks	Yes						
Moveable Inspection	System	I	No				
Haunches Present ov	er Roa	dway	NO				
Utilities	N No	Utilities	presen	ıt			

PROPOSED IMPR	OVEMENTS
(75A) Type of Work Proposed	35 - Rehabilitation -
(75B) Work Done By	Deterioration 1 - Work to be done by
(76) Length of Structure Improvement	contractft.
(94) Bridge Improvement Cost	\$
(95) Roadway Improvement Cost	\$
(96) Total Project Cost	\$ 30000
(97) Year of Improvement Estimate	2014
(114) Future ADT	59131
(115) Year of Future ADT	2038
DOT Bridge Program List No	27
Project No	0096-0201
Advertised Date	09/18/2019

- ---

- LOAD RATING & POSTING

(31) Design Load	5 - HS 20
(63) Operating Rating Type	1 - Load Factor (LF)
(64) Operating Rating	62.3
(65) Inventory Rating Type	1 - Load Factor (LF)
(66) Inventory Rating	37.4
Evaluation Code	L - Load Factor
Year of Evaluation	2011
(70) Bridge Posting	5 - Equal to or above legal loads
(41) Structure Status	A - Open

(41) Structure Status

INSPECTOR'S SIGNATURES:

1)	Mielel O'Ham	Date:	09/26/2018	P.E. SIGNATURE:	J Massin	Date:	10/02/2018
2)	Matthew Rusenburg	Date:	10/01/2018	P.E. #		- 	10/22/2018
3)	Machen figuore	Date:	10/01/2018		AnyEstab		10/22/2018
4)		Date:					

FIELD INSPECTION REPORT

Leastion				1050		Crear		
Location: Main Material:	OVER HOUSAT		Year Built:	1953		-	er Required:	
	4 - Steel continu		Year Rebuilt	1978		3100p	er Used:	\checkmark
Main Design:	03 - Girder and	Floorbeam						
Inspectors:					<u>Visits:</u>			
Lead Inspector	:	Matt Ri	esenberg		Visit Dat	e: Temp:	Start Time:	End Time:
Inspector: Task:					09/10/20	018 65	08:00 AM	04:00 PM
Area, 12		BSE - I	Vanager		09/11/20)18 71	09:00 AM	02:45 PM
Liguore, Matt		BSE - I	nspector					
O'hara, Mike		BSE - I	nspector					
Riesenberg, Ma	att	BSE - I	nspector					
58. DECK:								
	inforced Concrete	e Deck with Bitur	minous Concre	te Overlay				Overall Rating: 5
	Rating							
	Overlay: 7	The bituminous	concrete overl	ay has the	following	deficiencies	5:	
Deck - S	Str. Condition: 5	 Span 4 in the 6" long x 4" wid (See Sketches) The underside Areas of scale 	2 have random Left Lane near le x 1-1/2" deep 2 - 6 and Photo of the reinforce e up to 1/4" dee ne transverse o r honeycomb. aunches have v	longitudina Abutment 2 o with exposi- os 6 & 7) d concrete p. cracks with vertical hair	2 has an sed deck deck has and with line crack	area of map (See Photo the followin out effloresc	cracking 5' lo 7). g deficiencies	wide in the left shoulder. ong x 3' wide with a pothole s: re and rust stains.
		rust stains. • Span 3 at norr with efflorescer • Span 4 Girder exposed rebar.	th side of Floor nce stalactites. 2 Deck Haunc increased from	Beam 19 b h north of F a 4 to a 5,	etween S Floor Bea with no r	Stringer 3 an Im 23 has a major deficie	d Stringer 4 h 20" long x 3" l ncies found th	efflorescence, moisture and has a 3' diameter damp area high x 1" deep haunch with hroughout. The 2016
		(See Sketches						
	Curbs: 7	Sloped granite • Scrapes with • Chipped edge Average Curb F Left (West): 2- Right (East): 2	rust stains. es up to 1' long Reveals: 1/2"	Ū	eficiencie	es:		

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

	(See Sketches 2 - 6 and Photo 14)
Median: N	
idewalks: 5	There is a sidewalk along the Right (East) side that has the following deficiencies:
	 Random areas of moderate scale up to 1/2" deep. Random transverse hairline cracks. Ponding water due to clogged scupper grates and scale areas. Debris accumulation up to 2" high. Span 2 Sidewalk near Pier 2 - 15' long x 3' wide hollow area with a 5' long x 2' wide x up to 2" deep area of severe scale with exposed rebar. Span 4 Sidewalk near Mid-Span - 76" long x 43" wide hollow area with a 20" long x 12" wide x up to 1-1/2" deep area of scaling with exposed rebar (See Photo 12)
	(See Sketches 2 - 6 and Photos 11 - 13)
Parapet: 6	Reinforced concrete parapets and Right (East) fence base have the following deficiencies:
Railing: 6	 Areas of light to moderate scale. Impact scrapes. Random horizontal and vertical cracks up to 1/8" wide with and without efflorescence. Spalls up to 18" long x 12" high x 3" deep with some undermining fence / railing post bases and exposed anchor bolts. Some spalls have adjacent hollow areas (See Photos 15 - 17). Right Parapet in Span 1 has a 4' long patch on the top face. Left Parapet in Span 4 has a spall with wood form left in place. Approach Parapets and Bridge Parapets are misaligned up to 1-3/4" (See Photos 18 & 19). There is a sign mounted on the Left Parapet in Span 2 with the following deficiencies: Missing washers at the base plate. Northwest connection bolt is loose. Sign panel has collision damage. (See Sketches 2 - 6 and Photos 14 - 19)
Railing: 6	 Single aluminum railing mounted on top of the parapets have the following deficiencies: Random collision scrapes. Left Rail in Span 2 has a 3" long x 1" wide tear (See Photo 20). Right Rail Post in Span 2 near Pier 2 is undermined due to a spall exposing two anchor bolts. Span 4 Left and Right Rail Posts have isolated locations of loose and missing anchor bolt nuts (See Photo 21). (See Sketches 2 - 6 and Photos 20 & 21)
Paint: N	
Fence: 7	 There is a 6'-3" high anodized aluminum fence at the Right (East) Fascia mounted on a 1' high concrete base. The fence has the following deficiencies: The fence is bent at Abutment 2. Refer to 'Parapet' item above. (See Sketches 2 - 6 and Photos 11 & 22)
Drains: 5	Scuppers and sidewalk drains have the following deficiencies:
	 Roadway scuppers have debris in the grates and basins, but all pipes were clear. Span 2, right shoulder scupper grate is 50% clogged with debris and has light vegetation growth. All four sidewalk drains were clogged at the time of inspection.

All four sidewalk drains were clogged at the time of inspection.

Form: BRI-18, Rev. 1/14	itical,Routine Bridge No: 01218	Town: NEWTOWN
Inspection type: Fracture Cr	Carried: INTERSTATE 84 EASTBOUND	
Inspection Date: 9/10/2018 Inspected by: Infrastructure E	Engineers	Crossed: HOUSATONIC RIVER Inventory Route: NHS
	-	-
	 There are a total of five broken / missing / disconnected east fascias in Spans 2-4. 	ed scupper pipe support U-poits at the west and
	• One of two nuts is missing at a scupper U-bolt at the	northwest corner of Span 4.
	PVC Weep pipes have the following deficiencies:	
	Span 1 at the southeast corner of interior Panel 1 in S broging guaget plots	Span 1 has a weep that drains onto a lateral
	bracing gusset plate.Span 3, west overhang near Floor Beam 20 has a part	rtially broken weep that does not leak onto
	superstructure elements.Weep pipe at east overhang drains onto Pier 3.	
Lighting Standard N	(See Sketches 2 - 25 and Photos 23 & 24)	w that is not fully tightoned
Lighting Standard: N	 Span 1, Left (West) Parapet junction box has (1) scre Span 1, Right (East) Parapet junction box cover has in 	
	 Span 2, Left (West) Parapet junction box is missing (2 Left (West) Parapet in the North Approach has a junc 	
	screws missing (See Photo 25).	
	(See Sketches 3 - 6 and Photo 25)	
Overall Utility Condition Rating	N - Not Applicable	
Utility Type/Size		
N No U	tilities present	
-		
Construction Joints: 8		
Expansion Joint: 7	The strip seal joints with concrete headers have the fol	lowing deficiencies:
	 Moderate to heavy accumulation of sand and debris i Concrete headers have random transverse hairline cr 	
	(See Sketches 2, 3 & 6 and Photos 26 & 27)	
Haunches Present over travelwa	ay? NO	
APPROACH CONDITION:		
Bituminous Approac	ch Pavement	Overall Rating: 8
Rating		
Approach Slab: 8	Rating is based on the condition of the approach pave	ment.
	Refer to 'Approach Pavement' item below.	
Relief Joints: N		
Approach Guide Rail: 6	Approach guide rails at all four corners have the follow	ing deficiencies:
	• Northwest approach metal beam guide rail has (1) of	(3) backed off anchor bolt nut at the parapet end
	anchorage (See Photo 29).Southeast approach concrete barrier has a 10" x 10"	x 1-1/2" deep spall (See Photo 30).
	(See Sketcher 2, 2, 8, 6 and Photor 28, 20)	
Approach Pavement: 8	(See Sketches 2, 3 & 6 and Photos 28 - 30) The bituminous approach pavement has the following	deficiencies:
	Paving seams are slightly open.North Approach in the Left Shoulder near bridge has	several longitudinal cracks.
	(See Sketches 2, 3 & 6 and Photo 30)	

inspected by: initiastructure i	Lingineers	inventory Route: And
Approach Embankment: 7	 Well vegetated. Southeast Approach Embankment has minor erosion. (See Sketches 2, 3 & 6) 	
Trafic Safety F		
	Less than 42" high solid concrete parapet on NHS.	
	Not R-B 350 compliant (Concrete parapet is 3" higher ab	oove approach rail).
	Not R-B 350 compliant.	
Approach Guardrail Ends: 1	Continuous with highway rail.	
59. SUPERSTRUCTURE:		
	gers and Floor Beams	Overall Rating: 4
Rating		
Bearing Devices: 6	 Rocker Bearings at the Abutments, Pier 1 and Pier 3. Ab mode at 65 degrees Fahrenheit. Abutment 2 and Pier 3 Bearings were in contraction mode deficiencies were noted: Random areas of peeling paint and light rust. Light to moderate laminar rust below rockers. Abrasion rust at the pins. Abutment 1 Anchor Bolt Nuts have up to +/-90% sectio Abutment 2, Girder 1 Anchor Bolt nuts have up to 50% Girder 1 Bearing at Abutment 1 has up to 5/16" pack ru Abutment 2 Bearings have heavy debris on the mason Fixed rocker bearings at Pier 2 have no significant deficient Stringer plate / block bearings have the following deficient Random bearings have up to 3/16" high gaps and / or pthe bearing plates and stringer bottom flanges and / or fle Isolated anchor bolts are not snug at the interior stringer Random bearings have light abrasion rust. 	de at 65 degrees Fahrenheit. The following n loss (See Photo 32). section loss. Ist between rocker and masonry plate. ry plate from joint repair. encies. ncies: back rust (typically 1/8") between the edges of oorbeam top flanges. er bearings. en the stringer bearing plates and floor beam
Stringers: 5	 Stringers have the following deficiencies: Stringer bottom flanges have minor waviness. Up to 1/4" vertical misalignment and / or 1/4" pack rust Fascia stringer bottom flanges have up to 3/16" x full w the floorbeam cantilevers resulting in up to 25% flange lo Span 3, Stringer 8 Bottom flange has 1' long x 4" wide Beam 21 and Floor Beam 22 resulting in 11% flange loss Span 3, Stringer 8 east side of web at Floor Beam 22 e 1/4" deep loss at the base of the web resulting in less that loss in bearing. 	idth section loss (3/4" original thickness) over oss. section loss to 1/2" remaining between Floor s (No change since 2016 Inspection). ast cantilever has a 10" long x up to 2" high x

(See Sketches 7, 9, 12 & 14 - 25 and Photos 38 & 39)

Girders: 5 Girders have the following deficiencies:

nspected by:	Infrastructure E	Inventory Route: NHS
		 Areas of peeling paint and light to moderate rust (See Photo 43). Webs have random welded repair plates at locations where steel coupons were removed. Welds are poor at these locations. Splices have light to moderate rust (See Photo 41). Web stiffeners and girder bottom flanges have random minor bends, scrapes, and gouges from construction. Girder ends at abutments have laminar rust. Girder ends have section loss in the bottom flange angles and end angle outstanding legs to 1/8" minimum remaining; non-critical locations. Girder bottom flange angle horizontal legs have random section loss up to 1' long x 7" wide x 3/16" deep (Span 1, Girder 1 near Floor Beam 5) and spotty areas of deeper loss up to 3" wide x 2" long x 5/16" deep (Span 4, Girder 2 near Floor Beam 27). Vertical legs of the bottom flange angles have isolated areas of up to full height x 1/4" deep section loss (Span 4, Girder 2 near Floor Beam 27). Bottom flange cover plates have areas of up to 3/8" deep section loss, typically not more than 3-1/2" wide, except at isolated locations where losses are up to full width x 1/4" deep (Span 4, Girder 2 near Floor Beam 27). Areas of up to 2" pack rust between the bottom flange cover plates and / or short bottom flange construction-fabrication plates. (See Photos 42, 44 & 47). Random web stiffeners have 3/16" deep losses at their bases. Girder 1 at Floor Beam 18 underside of bottom flange cover plate has a 3" long x 2" wide rusted through hole due to pack rust with a rust induced crack 1" long. The 1" long crack was not noted in the 2016 Inspection. Girder 1 and Girder 2 bottom flange construction / tab plates near Floor Beam 26 have pack rust, kniftedging and perforations up to 1" wide. Flange and web losses are estimated to be < 5% in all critical zones. (See Sketches 7, 9, 12 & 14 - 25 and Photos 40 - 49)
F	Toor Beams: 4	 Floor Beams and Floor Beam Cantilevers have the following deficiencies: Areas of peeling paint with exposed primer and/or light to moderate rust. Stiffeners and flanges have random mis-drilled holes and bends and / or gouges throughout. Random top flange tie plates are in contact with the girder webs at the tie plate pass-throughs. Interior Floor Beams: Webs have random welded repair plates at locations where steel coupons were previously removed. Welds are poor at these locations with areas of surface rust (Refer to 'Welds-Cracks' item below) (See Photo 60). Flanges and stiffeners have minor bends from construction. Webs and flanges have areas of heavy rust with random heavy rust at center line of bridge. Hatches in the first intermediate floor beams from the abutments have been welded shut. The end floor beams at the abutments have areas of heavy to laminar rust with section loss along the top and bottom flanges. The 3/4" original flanges have areas with as little as 3/8" remaining on the horizontal legs, with losses extending into the vertical legs. Bottom flange losses in the end floor beams are estimated to be up to 13% (Floor Beam 1) near midspan with slightly greater losses near the supports at isolated locations. End floor beam top flange losses do not exceed 14% (Floor Beam 1). The bases of the web stiffeners of the end floor beams have laminar rust, knife edging, and perforations up to 2" long x 1" high. Interior floor beams flanges have areas of laminar rust near midspan of the floor beams with section losses up to 3/16" deep, resulting in up to 6% top flange loss (worst at Floor Beams 6,14 & 18) and 10° bottom flange loss (worst at Floor Beam 4). Webs near mid-span of the floor beams have areas laminar rust with up to 3/16" deep section loss.

Form: BRI-18, Rev. 1/14 Inspection type: Fracture Critical,Routine Inspection Date: 9/10/2018 Inspected by: Infrastructure Engineers

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	 Knee braces have up to 3/4" thick pack rust between plates at the girder bottom flanges.
	 Web stiffeners with up to full width x 4" high x 3/16" deep section loss. Cantilever bottom flange angle horizontal legs with up to 5-1/2" wide x 3" long x 3/16" deep section loss.
	adjacent to stiffener bases.
•	• Cantilever bottom flange angle horizontal legs between Stringer 7 & Stringer 8 have areas if up to 1'
	long x 4" wide x 1/8" deep section loss and the vertical legs have up to full height x 2" long x 1/8" deep
	section loss. • Cantilever ends with up to 5/8" thick pack rust between clip angles and gusset plates.
•	Random areas of moderate accumulation of bird debris on top flanges.
	• Rivet end plates (outside faces) have pack rust 1" thick and heavy rust.
	• Bottom flange angles have losses up to 3/16" deep at isolated areas, resulting in up to 15% flange loss (Floor Beam 18, Left (West) Cantilever).
	• Top flange angles have losses that are up to 3/16" deep at isolated areas, resulting in up to 11%
	section loss. • Wab bases have up to 2/46" door contian loss (Floor Boom 2. Dight (Foot) Contilever) and isolated 2".
(Web bases have up to 3/16" deep section loss (Floor Beam 3, Right (East) Cantilever) and isolated 2" diameter x 1/4" deep section loss with pinholes at the Floor Beam 1, Right (East) Cantilever web. Isolated full width x 1" high rusted through holes at Floor Beam 1 Web Stiffeners under Stringer 1.
I	Lateral Bracing and Gusset Plates:
•	 Up to 3/4" pack rust at the connections to the gusset plates.
	• Random areas of up to 1/8" section loss on bracing members, typically along the center line of the bridge.
	 Bracing members are bowed, sag, or bent upwards up to 4".
	 Gusset plate at Girder 2 and Floor Beam 1 has up to 1/4" section loss at the gusset plate due to leakage from a weep above.
	(See Sketches 7 - 29 and Photos 50 - 64)
Trusses - General: N	
Trusses - Portals: N	
Trusses - Bracing: N	
Paint: 7	 Less than 10% of painted surfaces have deterioration.
Rust: 4	Refer to items above.
Machinery Movable Span: N	
	 Random missing rivets along the superstructure, including girder web splices near Floor Beam 12 and Floor Beam 18 that have up to four (4) missing rivets. Random rivets are loose, not properly seated, and/or have abrasion rust. Random rivet heads have up to 50% section loss. There are also random loose catwalk bolts (Refer to 'Catwalks item below).
	Main girders and floor beams are considered to be fracture critical. Rivets in tension zones are fatigue
	category - D'.
	Welds throughout the structure had the following deficiencies:
	• There are random tack welds along the superstructure, including tack welds along the floor beam tie plates.
·	• Girder and floor beam webs have random areas where steel coupons were removed in the past and
l l l l l l l l l l l l l l l l l l l	steel plates were welded in place. These coupon welds are typically of poor quality and have random painted over slag. Previously noted cracks in the coupon welds at Floor Beam 15 in Span 3 and Girder 2 near Floor Beam 27 were not found during this inspection (See Photo 60).
Timber Decay: N	
Concrete Cracking: N	
	 Isolated stringer bottom flanges, floor beam flanges, random web stiffeners, lateral bracing members / plates and girder bottom flanges have random minor bends, dents, scrapes, and gouges from construction.

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Member Alignment: 6	 Random Floor Beam top flange tie plates are in contact with the girder webs at the tie plate pass-throughs. Random bolt holes are misaligned. There are random gaps at the stringer bearing plates (Refer to 'Bearings' item above). Up to 1/8" vertical misalignment and / or 1/8" pack rust at the bolted splices (Refer to 'Stringers' item above). Lateral bracing members have random bowed, sagging, and/or twisted members (Refer to 'Floor Beams' item above).
Deflection Under Load: N	(N) - Normal; (E) - Excessive
Vibration Under Load: N	 (N) - Normal; (E) - Excessive Primary member vibration was normal. Catwalks and isolated lateral bracing members had moderate
	vibration.
Stand Pipes: N	
Catwalks: 6	 The catwalk was secure during the inspection. The following deficiencies were noted: Areas of peeling paint and light to moderate rust. Up to 3/4" pack rust between the catwalk tread plates and support channels. Catwalk tread plates have up to 3/16" deep section loss and areas of heavy rust. Handrails have random bends up to 1". There are isolated gaps between the clip angles and sidewalk support channels; as-built condition. Catwalk tread plate near Floor Beam 14 has a large area of heavy rust with a 1.5" diameter thin area with small rusted throughout holes. Three (3) loose bolts at a handrail connection between Floor Beam 22 & Floor Beam 23. (See Sketches 7, 9, 12 & 14 - 25 and Photos 66 & 67)
Movable Inspection System: N	
Barrel Ladders: N	
Are	e Barrel Ladders OSHA Compliant? NA

60. SUBSTRUCTURE:

Reinforced concrete	Reinforced concrete abutments, wingwalls and piers.	
Rating		
Abutments - Stem: 7	The abutment stems are significantly covered with fill mate	erial. The following deficiencies were noted.
	• Abutment 1 West Cheekwall has a 2' long x 6" high x 1" of shallow rebar.	deep spall on the step-out and a 6" diameter
	• Abutment 2 West Cheekwall has a 1' long x 7" wide x 3" face.	deep spall with exposed rebar on the inside
	(See Sketches 30 & 31 and Photos 68 - 69)	
Abutments - Backwall: 5	Abutment backwalls have the following deficiencies:	
	 Areas of heavy graffiti. Both backwalls have an uneven pour at the cold joint with throughout. 	h concrete overpour and edge spalls
	Abutment 1: • Random spalls up to 4-1/2' long x 8" high x 2" deep. • Isolated scale up to 1/4" deep. • Isolated 1/16" wide vertical crack.	
	 Abutment 2: Random spalls up to 18" long x 5" high x 1" deep with explored and the spalls up to 18" long x 5	posed rebar along the cold joint.

pected by: Infrastructure I	Engineers Inventory Route: NHS
	• Backwall near Girder 2 at the top has a 5' x 15" hollow concrete patch area and an adjacent spall up 3' long x 1' high x 3" deep.
	(See Sketches 30 & 31 and Photos 68 - 71)
Abutments - Footings: N	Not visible.
Abutments - Settlement: 8	None noted.
Abutments - Wingwalls: 6	Wingwalls have the following deficiencies:
	 Random hairline cracks. Isolated hairline map cracks. Random hollow areas up to 2' diameter. Isolated small spalls and pop-outs up to 6" diameter x 2" deep. Random joint fill material is deteriorated and / or missing.
	The following wingwalls have lateral / vertical misalignment along construction joint as noted below, no change since the last inspection:
	 Wingwall 1B is laterally misaligned 13/16" at the base and 1-3/4" at the top. Wingwall 2A parapet has vertical misalignment up to 1-3/4" at the top. Wingwall 2B has lateral misalignment up to 1/2" and is vertically misaligned by up to 1-1/8" at the top
	(See Sketches 6, 32 & 33 and Photos 72 - 74)
Piers/Bents - Caps: N	
Piers/Bents - Pile Bent: N	
Piers/Bents - Columns: 6	Solid concrete pier walls have the following deficiencies:
	 Random pop-outs Graffiti. Scale up to 1/2" deep. Spalls up to 1' high x 2' wide x 1" deep with exposed rebar. Horizontal and vertical hairline cracks up to 18' long with and without efflorescence. A few vertical cracks extend into the top surface of the piers. Pier 1, South Elevation and Pier 3, North Elevation have isolated hollow areas up to 1' diameter (See Sketches 34 -36 and Photo 75)
Piers/Bents - Footings: 7	Refer to 'Erosion-Scour' item below.
Piers/Bents - Settlement: 8	None noted.
Erosion - Scour: 6	 Erosion - Rating = 6 Scour - Rating = 6 Per Underwater Inspection report by Infrastructure dated 07/18/2018: Pier 1 step footing exposed up to 32' long x up to full-height (5.9' high) along both faces of the pier. Pier 1 footing exposed up to 6.8' vertically (previously 5' high) over a length of 30' on the South face and 8' on the North face near the Upstream (West) end of the pier with no undermining. Pier 1 has exposed surfaces of the step footing and footing have abrasion up to 1/8" deep. Pier 2 step footing is exposed up to 26' long x 2.1' high (previously 2' high) on the South face of the pier and along the Upstream (West) Nose. Pier 2 footing has exposed surfaces of the step footings footing exhibit scaling up to 1/8" deep. Pier 2 footing has random chamfer spalls up to 6" long x 3" wide x 1" deep. Pier 3 has no exposure of the step footing or footing. Channel at the upstream nose has a minor localized scour.
	There are no significant changes to the exposed footing since the 2016 Inspection.
	• The channel bottom along Pier 1 primarily at the nose and North face has had scour up to 4.1' high

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	since the 2016 Underwater Inspection.
	• The channel bottom along Pier 2 North face has had scour up to 5.6' high since the 2016 Underwater
	Inspection.
	• The channel bottom along Pier 3 has remained relatively unchanged (less than 1.1' variations) since the 2016 Underwater Inspection.
	 The channel bottom along the up and downstream fascias have had random areas of mostly
	aggradation up to 6.5' high since the 2016 Underwater Inspection.
	(Refer to 2018 Underwater Report)
Concrete Crack - Spall: 5	See items above.
Steel Corrosion: N	
Paint: N	
Timber Decay: N	
Collision Damage: 8	
Debris: 6	 Light to heavy accumulation debris at the abutments / pedestals and pier seats.

61. CHANNEL AND CHANNEL PROTECTION:

Housatonic River

Rating	
Channel - Scour: 6	Refer to 'Erosion-Scour' item above.
Embankment - Erosion: 7	 There is no significant erosion along the channel embankments.
Debris: 6	Per Underwater Inspection report by Infrastructure dated 07/18/2018:
	 There are three 30' long x 1' diameter fallen trees in the channel South of Pier 3. There is a 30' long x 12' high x 2' diameter area of timber debris on the Upstream (West) nose of Pier 1. (See 2018 Underwater Inspection Report)
Vegetation: 7	
vegetation. 7	The channel embankments are well vegetated. (See 2018 Underwater Inspection Report)
Channel Change: 6	Per Underwater Inspection report by Infrastructure dated 07/18/2018:
	 There is no apparent change to the channel orientation. Bridge No. 04180 is located approximately 40' Upstream (West) of the bridge. There is approximately a 150' long x 20' wide x 3' high sandbar/area of aggradation located northwest of Pier 3 near Bridge No. 04180.
	(See 2018 Underwater Inspection Report)
Fender - System: N	
Spur Dikes and Jetties: N	
Rip Rap: N	

62. CULVERTS AND RETAINING WALLS:

Overall Rating: N

Overall Rating: 5

Rating	
Barrel: N	
Concrete: N	
Steel: N	
Timber: N	

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Headwall: N	
Cutoff Wall: N	
Debris: N	
Retaining Wall System: N	
Footing: N	

LOAD POSTING:

<u>Rating</u>	
Single Unit (Tons):	
Semi Trailer (Tons):	
4 Axle (Tons):	
3S2 (Tons):	
All Vechicles:	
Advanced Warning:	
Warning At Bridge:	
Legibility:	
Visibility:	

VERTICAL CLEARANCE POSTING

Min. Vert Under Clearance:	Ft	In			
Posted Clearence Under Bridge:	Ft	In			
Posted Clearence On Bridge:	Ft	In			
Advanced Warning:					
Warning At Bridge:					
Legibility:					
Visibility:					

NOTES / COMMENTS:

Character of Traffic: Heavy volume of mixed weight traffic.

Additional Notes:

- Bridge Identification Number is clear and legible.

- Bridge is logged from west to east, but items are labeled south to north with Girder 1 and Stringer 1 at the Left (West) fascia which is consistent with previous reports and bridge plans.

- Bridge was inspected using catwalk, 60' snooper, rope access, safety boat, trooper and shoulder closures on I-84 Eastbound.

Additional Comments:

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

National Bridge Elements Inspection type: Fracture Critical,Routine Inspection Date: 9/10/2018 Inspected by: Infrastructure Engineers

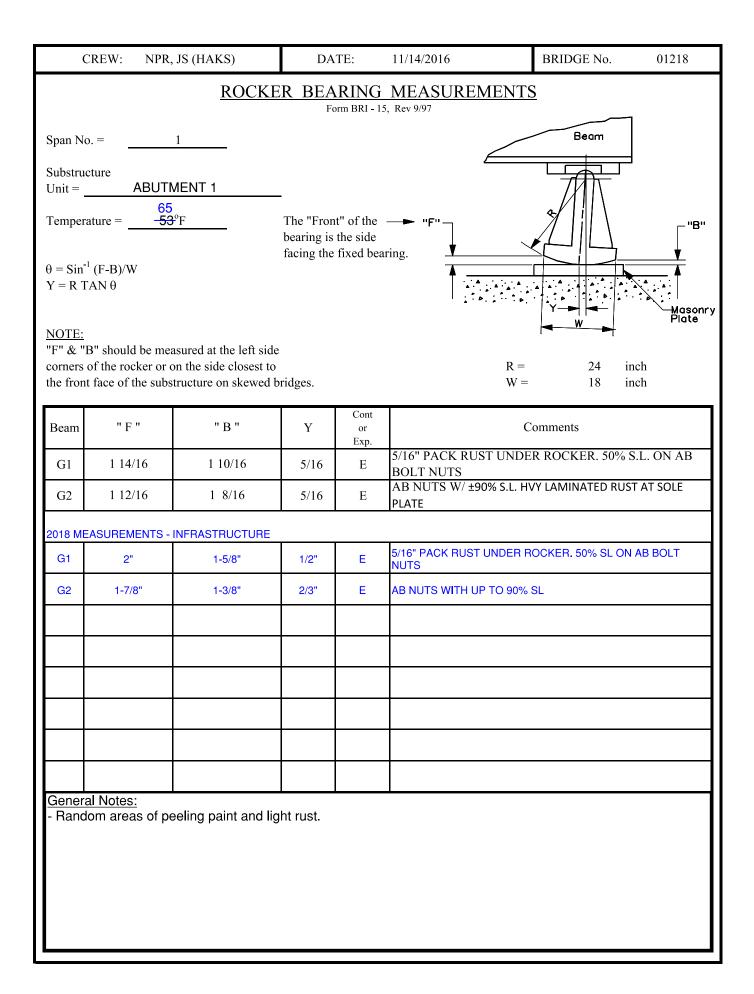
	Environment	Total Quantity	Units	Condition State 1	Condition State 2	Condition State 3	Condition State 4
12 - Reinforced Concrete Deck	Mod.	55519	sq. ft.	49634	5881	4	0
1080 - Delamination/Spall/Patched Area		77		0	73	4	0
1120 - Efflorescence/Rust Staining		800		0	800	0	0
1130 - Cracking (RC and Other)		5008		0	5008	0	0
510 - Wearing Surfaces	5	47520	sq. ft.	47419	100	1	0
3210 - Delamination/Spall/Patched Area/Pothole (Wearing Surfaces)		1		0	0	1	0
3220 - Crack (Wearing Surface)		100		0	100	0	0
107 - Steel Open Girder/Beam	Mod.	1575	ft.	1531	2	42	0
1000 - Corrosion		42		0	0	42	0
1900 - Distortion		2		0	2	0	0
515 - Steel Protective Coating		61600	sq. ft.	61600	0	0	0
113 - Steel Stringer	Mod.	6273	ft.	6188	80	5	0
1000 - Corrosion		80		0	75	5	0
1900 - Distortion		5		0	5	0	0
515 - Steel Protective Coating		44352	sq. ft.	44352	0	0	0
152 - Steel Floor Beam	Mod.	1942	ft.	1837	3	102	0
1000 - Corrosion		102		0	0	102	0
1900 - Distortion		3		0	3	0	0
515 - Steel Protective Coating		32175	sq. ft.	29924	1608	643	0
210 - Reinforced Concrete Pier Wall	Mod.	167	ft.	152	15	0	0
1080 - Delamination/Spall/Patched Area		2		0	2	0	0
1090 - Exposed Rebar		4		0	4	0	0
1190 - Abrasion/Wear (PSC/RC)		9		0	9	0	0
215 - Reinforced Concrete Abutment	Mod.	141	ft.	136	5	0	0
1080 - Delamination/Spall/Patched Area		3		0	3	0	0
1090 - Exposed Rebar		2		0	2	0	0
300 - Strip Seal Expansion Joint	Mod.	120	ft.	120	0	0	0
2350 - Debris Impaction	1	0		0	0	0	0
2360 - Adjacent Deck or Header		0		0	0	0	0
311 - Movable Bearing	Mod.	8	each	0	8	0	0
1000 - Corrosion	1	8		0	8	0	0
515 - Steel Protective Coating		96	sq. ft.	96	0	0	0
313 - Fixed Bearing	Mod.	2	each	0	2	0	0
1000 - Corrosion		2		0	2	0	0
515 - Steel Protective Coating		24	sq. ft.	24	0	0	0
330 - Metal Bridge Railing	Mod.	1584	ft.	1579	4	1	0
1020 - Connection		4		0	4	0	0
7000 - Damage	-	1		0	0	1	0
331 - Reinforced Concrete Bridge Railing		1584	ft.	1491	90	3	0
1080 - Delamination/Spall/Patched Area		10		0	7	3	0

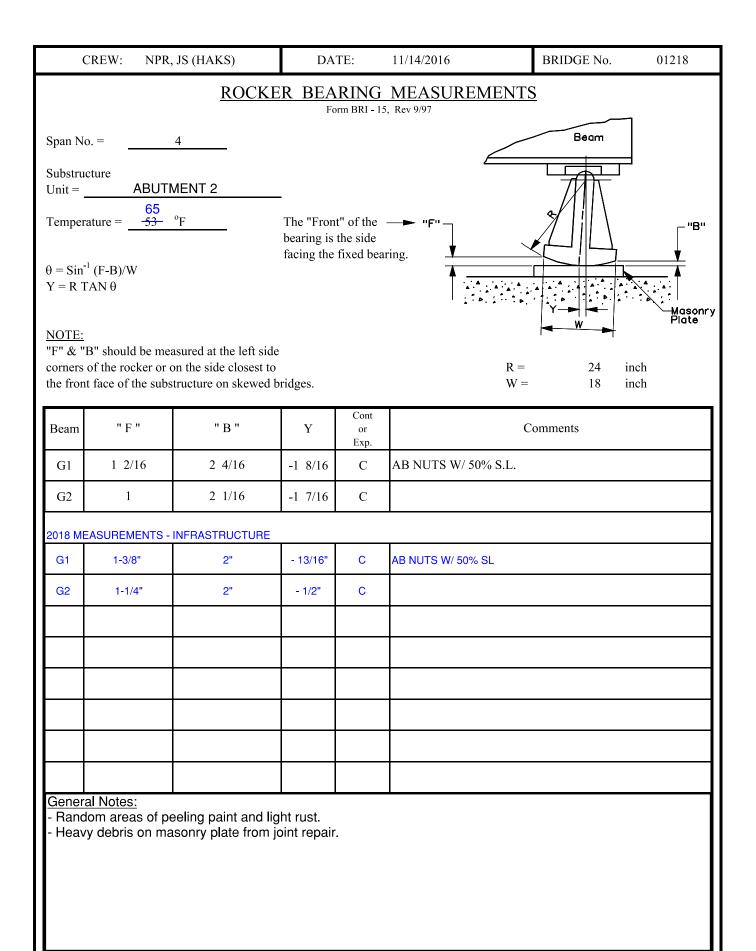
National Bridge Elements

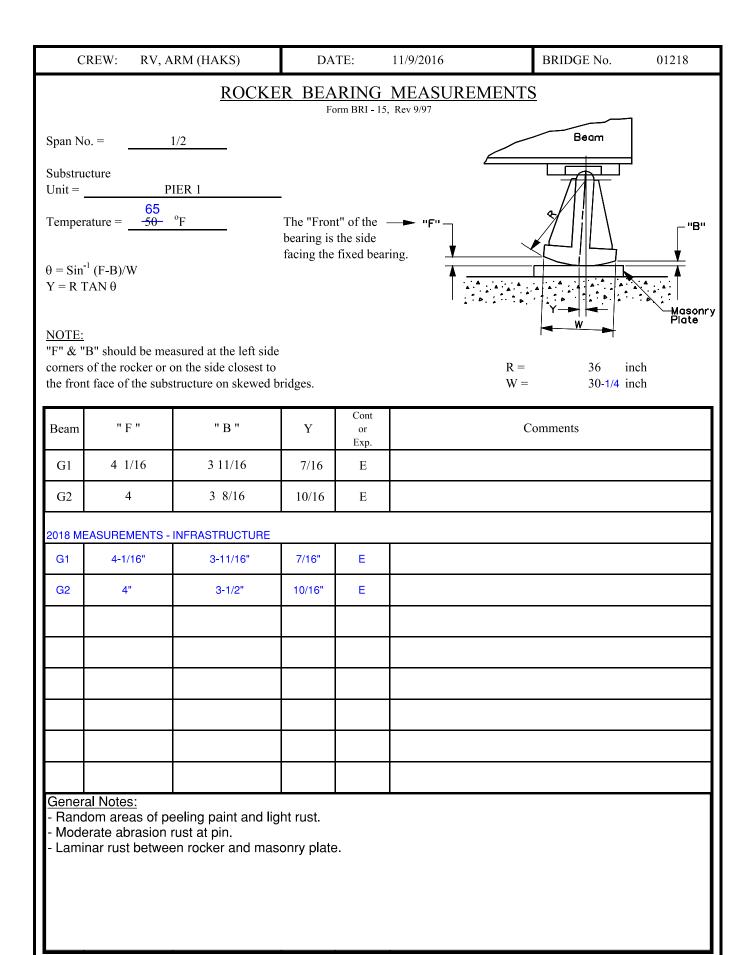
Inspection type: Fracture Critical,Routine Inspection Date: 9/10/2018 Inspected by: Infrastructure Engineers

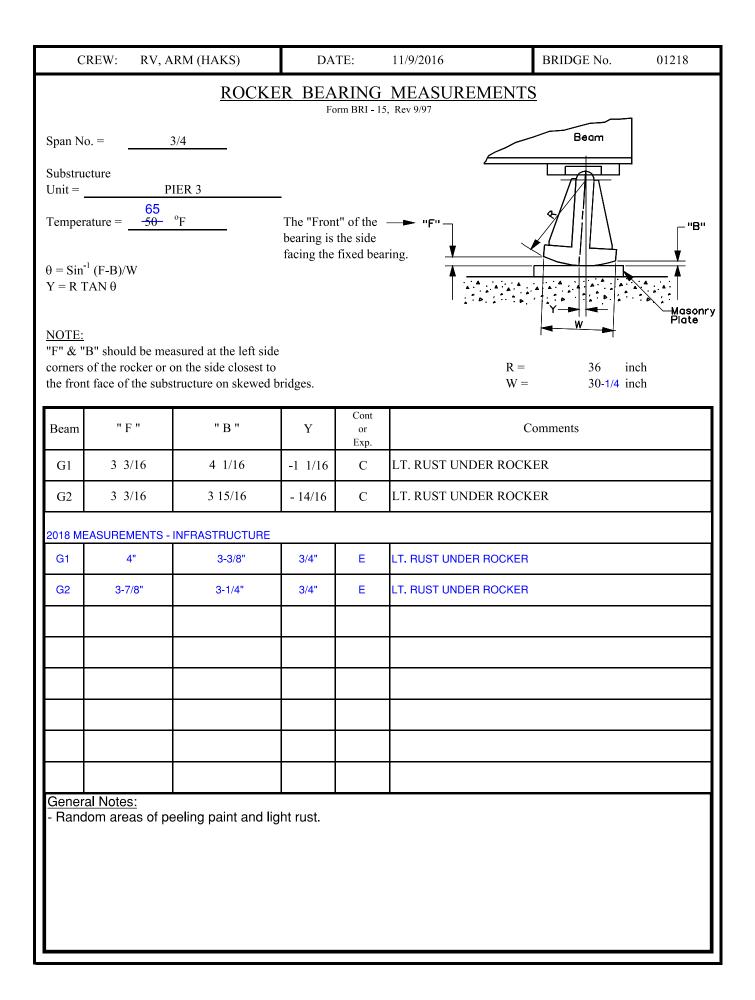
Bridge No: 01218

1090 - Exposed Rebar	3	0	3	0	0
1130 - Cracking (RC and Other)	80	0	80	0	0





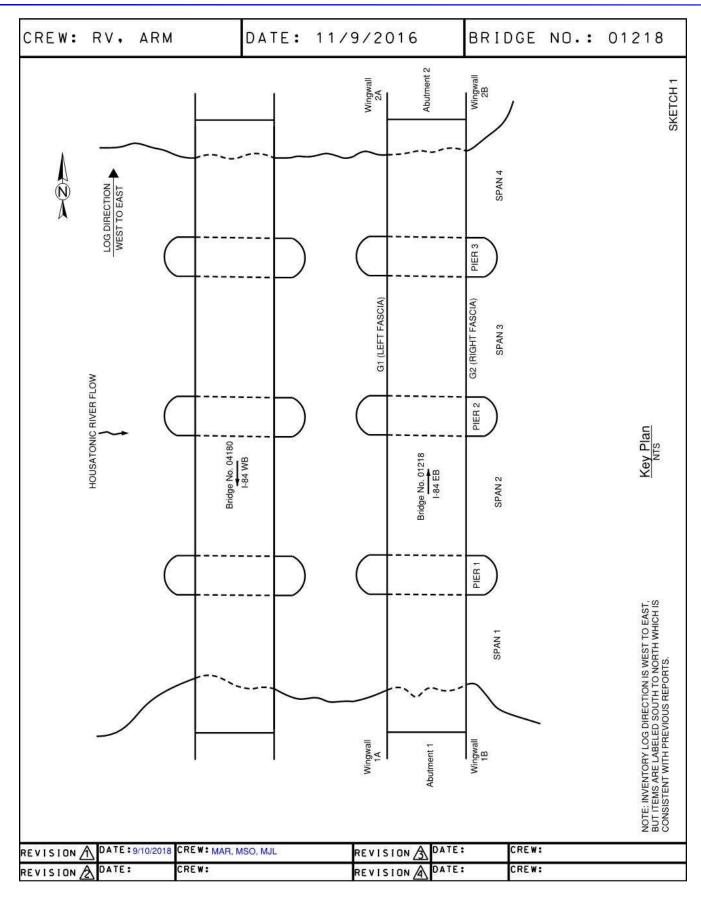




Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

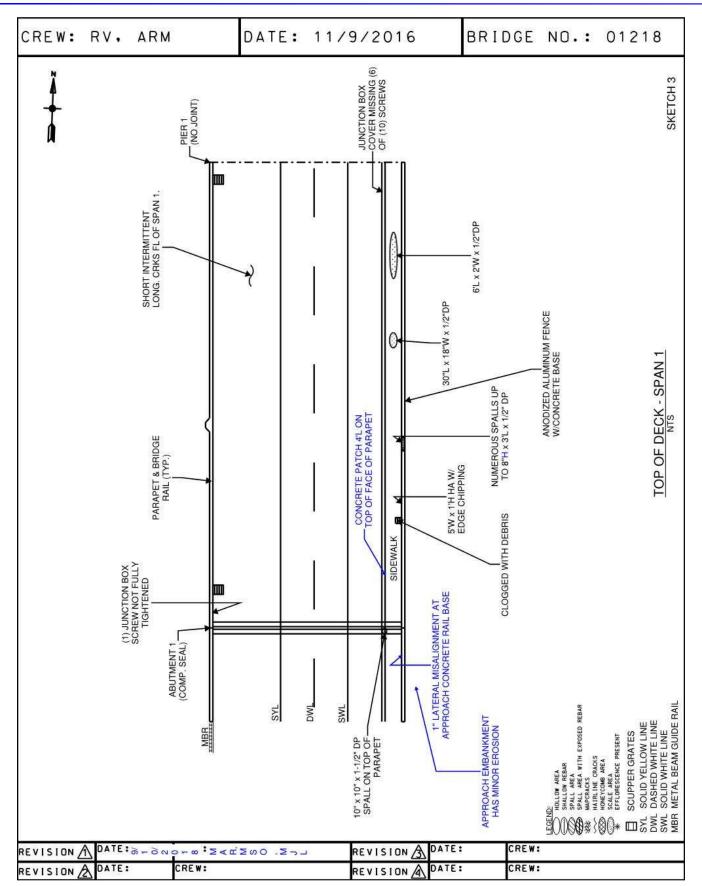
FRACTURE CRITICAL MEMBERS / FRACTURE PRONE DETAILS

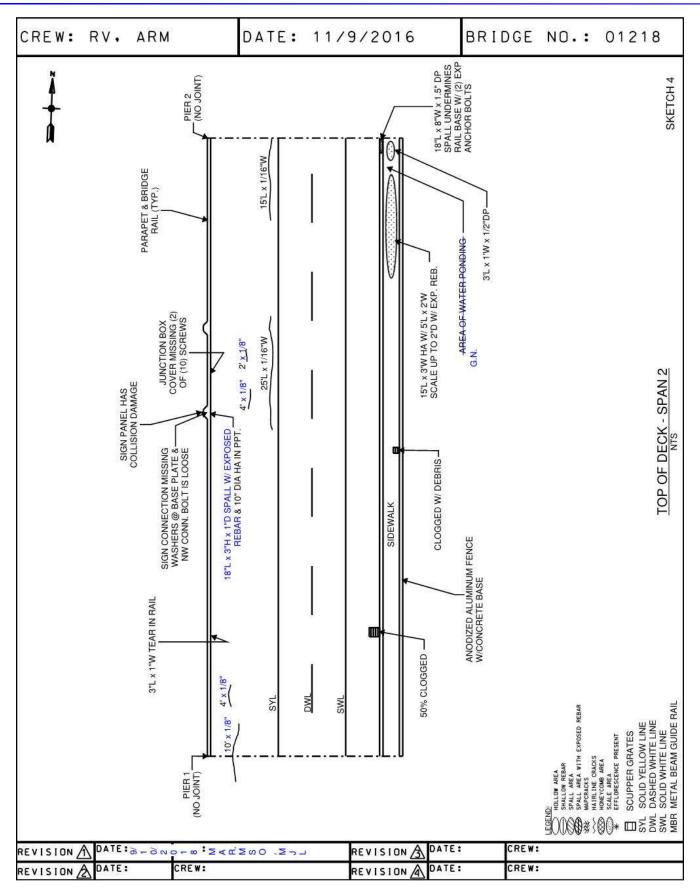
Inspectors:		<u>Visits:</u>						
Lead Inspector:	Matt Riesenberg	Visit Date: Temp: Start Time: End Time:						
nspector:	Task:	09/10/2018 65 08:00 AM 04:00 PM						
Area, 12	BSE - Manager	09/11/2018 71 09:00 AM 02:45 PM						
₋iguore, Matt	BSE - Inspector	_						
D'hara, Mike	BSE - Inspector	_						
Riesenberg, Matt	BSE - Inspector							
Fracture Critical Inspection Frequency: 24 Months Fracture Critical Type Code: D Two Girder System, riveted / bolted plate girders								
Structure Type: Highway	Bridges Year Built: 1953 ADT: 39795	5 Year of ADT: 2018 % Truck: 9						
	ded: 60' Snooper & Catwalk							
Traffic Control Required	I: Shoulder closure on I-84 EB							
Reference to Plans:	Project No. 96-49 (1950) & 96-115 (1975)							
	MEMBER/DETAIL	. TYPE # 1						
Member/Details Type:	A One or two steel girder systems	Fracture Critical: Yes						
Fatigue Category:	D Steel Type: A-588	Fatigue Prone: Yes						
		-						
Description:	Riveted girders & floorbeams are fracture critica	al. Rivers in tension zones.						
Inspection Procedure:	100% hands-on.							
Condition Comments:	See BRI-18.							
Procedure Followed This Inspection? Yes If No please explain:								

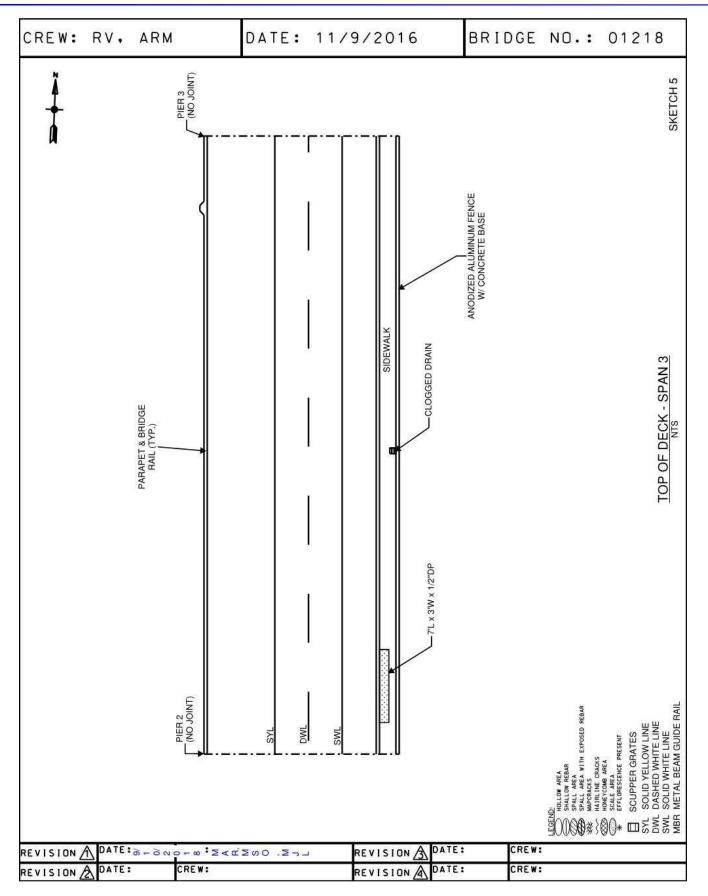


Town: NEWTOWN **Carried:** INTERSTATE 84 EASTBOUND **Crossed:** HOUSATONIC RIVER Inventory Route: NHS

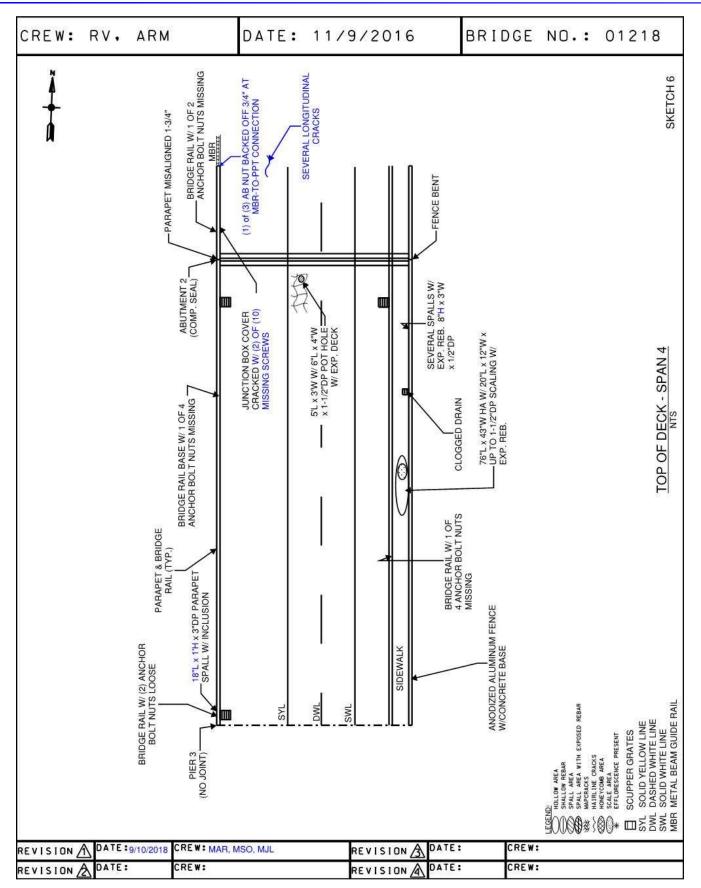
CREW: RV, ARM	DATE:	11/9/2016	BRIDGE	NO.:	01218
	TOP OF DE	CK - GENERAL NOTES			
OVERLAY					
PAVING SEAMS ARE OPEN SLIGHTLY.					
CURBS:					
 SCRAPES WITH RUST STAINS. 					
CHIPPED EDGES UP TO 1'LONG x 6"WIDE.					
LEFT (WEST): 2-1/2" RIGHT (EAST): 2-1/2"					
SIDEWALKS:					
RANDOM AREAS OF MODERATE SCALE UF	TO 1/2" DEEP LINI ESS	OTHERWISE NOTED			
RANDOM TRANSVERSE HAIRLINE CRACKS					
PONDING WATER DUE TO CLOGGED SCUP		LE AREAS.			
DEBRIS ACCUMULATION UP TO 2"D.					
PARAPETS:					
AREAS OF LIGHT TO MODERATE SCALE.					
IMPACT SCRAPES.					
· RANDOM HORIZONTAL AND VERTICAL CRA	ACKS OPEN UP TO 1/8"W	/IDE W/ & W/O EFFLORESCENCE.			
PARAPETS & FENCE BASE W/ RANDOM SP	ALLS UP TO 18" L x 12" F	H x 3" DP.			
SINGLE PIPE ALUMINUM RAILING:					
RANDOM COLLISION SCRAPES					
DRAINS:					
ROADWAY SCUPPERS HAVE DEBRIS IN GE	RATES & BASINS, BUT A	LL PIPES WERE CLEAR. •			
EXPANSION JOINTS (STRIP SEAL WITH CON	CRETE HEADERS):				
MODERATE TO HEAVY ACCUMULATION OF	SAND & DEBRIS IN THE	JOINT.			
CONCRETE HEADERS W/ RANDOM TRANS	VERSE HAIRLINE CRACK	KS.			
APPROACH GUIDE RAILS:					
METAL BEAM GUIDE RAILS AT SOUTHWES	T & NORTHWEST APPRO	DACH CORNERS.			
NORTHEAST APPROACH CONCRETE BARF	RIER HAS NO NOTABLE I	DEFICIENCIES.			
APPROACH PAVEMENT:					
• PAVING SEAMS ARE OPEN SLIGHTLY.					
APPROACH EMBANKMENT:					
WELL VEGETATED					
					SKETCH 2
EVISION A DATE : 9/10/2018 CREW: N	IAR, MSO, MJL	REVISION	CREW:		
EVISION A DATE: CREW:					







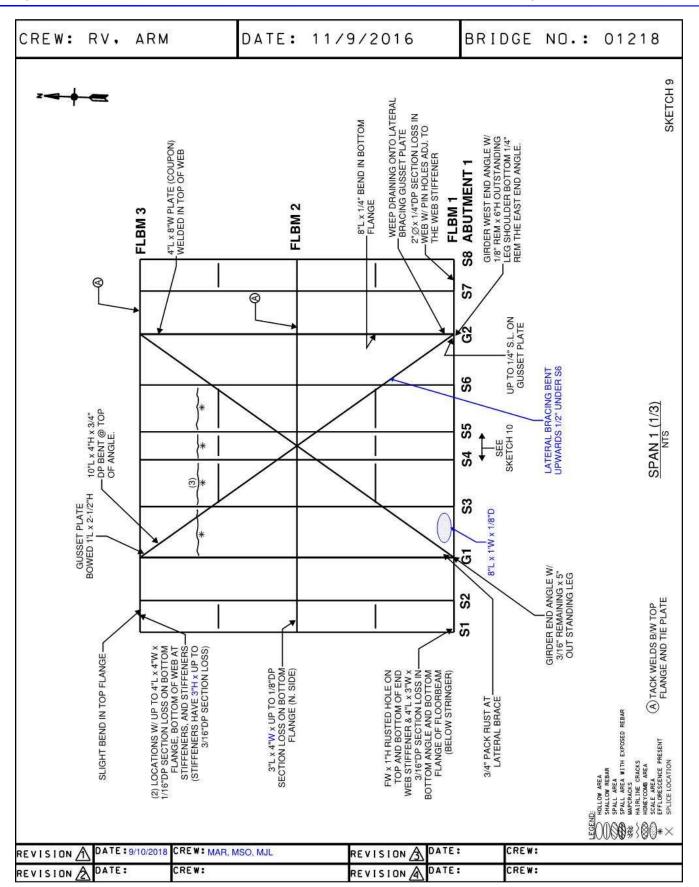
Bridge No: 01218



CREW: RV,	ARM		DATE:	11/	9/2016	BR	IDGE	NO.	:	01218
GENERAL:		GENE	RAL NOTE	S FOR	DECK & FRA	MING PLA	N			
RANDOM AREAS OF F	PEELING PA	INT W/ MODER	ATE TO HEAVY	RUST.						
SECTION LOSSES IN N										
• MINOR DEFECTS THA				R SERVICE		OR CATWALKS	VERE NOT	CARRIED	OVE	R FROM THE
PREVIOUS REPORT OR										
PVC WEEP PIPES EXT	END LATER	ALLY AWAY FF	OM FLANGES,	BUT DO NO	OT EXTEND BELOW	BOTTOM FLAN	IGES AT S	OME LOCA	FION	S. SOME WEEP
PIPES HAVE BEEN EXTE	NDED. ONL	Y WEEP PIPES	THAT DRAIN OF	NTO BRIDG	E ELEMENTS ARE	SHOWN IN THE	SKETCHE	S.		
UNDERSIDE OF DECK:										
AREAS OF SCALE UP	TO 1/4"D.									
RANDOM HAIRLINE TR	ANSVERSE	CRACKS W/ &	W/O EFFLORES	CENCE						
AREAS OF MINOR HON	VEYCOMB.									
GIRDER DECK HAUNC	HES HAVE	VERTICAL HAIR	LINE CRACKS.							
BEARING DEVICES:										
ROCKER BEARINGS A	T ABUTMEN	NTS AND PIERS	1 & 2 W/ AREAS	OF PEELI	NG PAINT AND LIG	HT RUST.				
FIXED TYPE ROCKER	BEARINGS	AT PIER 3 HAVE	NO NOTABLE	DEFICIENC	IES.					
STRINGERS & STRINGE	R BEARING	<u>S:</u>								
STRINGER BOTTOM FI	LANGES HA	VE RANDOM MI	NOR WAVINESS	S.						
RANDOM MINOR GAPS									1922	
RANDOM TACK WELDS			RINGER BEARIN	IG PLATES	AND FLOORBEAM	TOP FLANGES	RANDOM	PREVIOUS	LY CI	RACKED/BROKEN
TACK WELDS ARE SHOW										
FASCIA STRINGERS B					EAVY LAMINAR RU	JST W/ UP TO 3/	16" DP x FI	JLL WIDTH	SEC	HON LOSS OVER
THE FLOOR BEAM CANT										
RANDOM AREAS OF M UP TO 1/4" PACK RUST					2.					
	EDO									
RIVETED BUILT-UP GIRE		TO AVUE DAOLA								
GIRDER BOTTOM FLAI										FORIORIA
WEBS HAVE RANDOM SPLICES W/ LIGHT TO			AT LOCATIONS	WHERE S	IEEL COUPONS W	ERE REMOVED.	WELDS A	RE POOR A	н	ESE LOCATIONS.
WEB STIFFENERS AND			ANGLES HAVE	RANDOM	MINOR BENDS SCI		LIGES FRO		пот	ON
ISOLATED GIRDER EN				THAT DOWN	UNIVERSITY DENDER, OU	IN LO AND GO	GGEOTTIC	W CONOT	0011	
SECTION LOSSES AT I				ANDING LI	EGS (1/8" MIN. REM	IAINING), NON-0	RITICAL L	OCATIONS	e.	
CANTILEVER FLOOR BE	AMS:									
CANTILEVER KNEE BR		E UP TO 3/4" PA	CK RUST BETW	EEN PLAT	ES AT THE GIRDEF	R BOTTOM FLAN	IGES.			
WEB STIFFENERS W/ I	JP TO FULI	WIDTH x 4" HIG	H x 3/16" DEEP	SECTION	LOSS.					
CANTILEVER BOTTOM						16" DEEP SECT	ION LOSS	ADJACENT	TOS	TIFFENER
BASES.	. E		inte and on o					, and the second s		
CANTILEVER BOTTOM	FLANGE A	NGLE HORIZON	TAL LEGS BETV	VEEN STR	NGER 7 & STRING	ER 8 HAVE ARE	AS OF UP	TO 1' LONG	x 4"	WIDE x 1/8" DEEP
SECTION LOSS AND THE									337/01	and the second s
CANTILEVER ENDS W/										
RANDOM AREAS OF M										
• RIVET END PLATES (O										
										OVETOU 7
	9/10/2018	CREW: MAR, M	SO MIL			DATE:	CREW			SKETCH 7
			JU, MJL		REVISION	18/05/04/11/19/102	1.1.0.0.1.0.0.0.0.0	N)		
REVISION A DATE:	ģ	CREW:			REVISION	DATE:	CREW			

DESCRIPTION OF MEDICE DREPAIR PLATES AT LOCATIONS WHERE STEEL COUPONS WERE PREVIOUSLY REMOVED. WELDS ARE POOR AT THESE UCURIONS WITH AREA OF SUPRACE PLUST. 	CREW: RV, AI	RM	DATE:	11/9/2016	BRI	DGE	NO.:	01218
 THE TANDE INVIDED TREATER PATES AT LOCATIONS WHERE STEEL COUPONS WERE PREVIOUSLY REMOVED. WELDS ARE POOR AT THESI LOCATIONS WITH ARADS INTITALES AND STIFFELRES AND STIFFELRES AND STIFFELRES AND STIFFELRES AND STIFFELRES AREAS OF EURY TUST WITHOUT ON CONSTITUCTION. THANGES MAY EARDS OF HEARY TUST WITHOUT WITHOUT ON CONSTITUCTION. THORIZONTAL LOSS OF TLOOR BEAM PARANGES NEW: "LITTIF REMEMBIAN GUARIAUX THICKNESS - 34" AT MUREPOUS LOCATIONS. AREAS WITHOUT ON CONTROL OF BRIDDED TOTE. THANDEN DON CHANGE TRE PATES ARE PRANT TO BE IN CONTACT WITH GIRDER WEBS AT THE TRE PATE HARSE THROUGHS. THANDEN DON CHANGE TRE PATES ARE DEAT VERTICALLY AND/OR LATERALLY UP TO 3" AT RANDOM LOCATIONS. SOme MEMBERS HAVE MINOR TOKISMS. ANDOM ON PARANGE MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT NOT EXCESSIVELY. ANDON DIS MACING MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT OF EXCESSIVELY. ANDON ON PARANGE MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT NOT EXCESSIVELY. ANDON DIS MACING MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT NOT EXCESSIVELY. ANDON DIS MACING MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT NOT EXCESSIVELY. ANDON DIS MACING MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT NOT EXCESSIVELY. ANDON DIS MACING MEMBERS WERE VIBRATING AT THE TIME OF INSPECTION. BUT NOT EXCESSIVELY. ANDON DIS ANDON BROWN MEMBERS AND DAMBERS AND AT THE CENTERLINE OF BRIDDIC AND A TOWARD SOME PARELS HAVE LAMINATED. ANDON DIS ANDON MEMBERS WERE WIDH AND AND MEMBER ENDS AT THE CENTERLINE OF BRIDDIC AND A TOWARD ON STRONE ON ENDERS AND COMPANY AND THE THE CONTROL OS AT SOME LOCATIONS. ANDON DIS ANDON MEMBERS WERE PROMINE WITH AND THE VIENT HAVE WERE PROMINE SETTION LOSS AT SOME LOCATIONS. ANDON DIS AND MEMBERS WERE PROMINE WITH AND AND AND AND AND AND AND AND AND AND		GENERAL	NOTES FOR	R DECK & FRAMIN	G PLAN (CC	DNT.)		
RUST W. NEGLIGIBLE TO 11/16' DEEP SECTION LOSS. RIVET HEADS W/ UP TO 40% SECTION LOSS AT SOME OF THESE LOCATIONS. 4. GUSSET PLATES HAVE RANDOM BENDS, TYPICALLY LESS THAN 1*. MORE SEVERE BENDS WRE SHOWN IN SKETCHES. 5. CATWALK THAN PAINT WITH LIGHT TO MODERATE RUST. 9. UP TO 34" PACK RUST BETWEEN TREAD PLATES AND CATWALK SUPPORT CHANNELS AT SOME LOCATIONS. 1. UP TO 34" PACK RUST BETWEEN TREAD PLATES AND CATWALK SUPPORT CHANNELS AT SOME LOCATION LOSS AT SOME LOCATIONS. 1. OCTIVALK TEAD PLATES HAVE RANDOM HEAVY RUST, HEAVY SURFACE RUST AND/OR UP TO 3/16' DEEP SECTION LOSS AT SOME LOCATIONS. 2. RANDOM HAND RAILS HAVE UP TO 1'' BENDS. SKETCH 3 SKETCH 3 SKETCH 3 SKETCH 3 1. STATES: 21102/218 CREV: MAR. MSO. MJL 1. SPACE 1. SP	WEBS HAVE RANDOM WELD LOCATIONS WITH AREAS OF FLANGES AND STIFFENERS WEBS AND FLANGES HAVE HORIZONTAL LEGS OF FLO GREATER FLANGE LOSSES W RANDOM TOP FLANGE TIE H HATCHES IN THE FIRST INT LATERAL BRACING MEMBER LATERAL BRACING MEMBER TWISTING.	DED REPAIR PLATES SURFACE RUST. HAVE MINOR BENDS AREAS OF HEAVY RU OR BEAM FLANGES H VERE NOTED. PLATES APPEAR TO B ERMEDIATE FLOOR B S: RS ARE BENT VERTIC	AT LOCATIONS W S FROM CONSTRU JST W/ RANDOM H IAVE +/-11/16" REI BE IN CONTACT W BEAMS FROM THE CALLY AND/OR LA	HERE STEEL COUPONS V ICTION. HEAVY RUST AT CENTERL MAINING (ORIGINAL THICH TH GIRDER WEBS AT TH ABUTMENTS HAVE BEEN TERALLY UP TO 3" AT RAM	VERE PREVIOUSL INE OF BRIDGE. (NESS = 3/4") AT N E TIE PLATE PASS WELDED SHUT. NDOM LOCATIONS	Y REMOVE	B LOCATIONS.	AREAS W/
CATMALKS: 4 AFEAS OF PELING PAINT WITH LIGHT TO MODERATE RUST. 4 UP TO 3/4* PACK RUST BETWEEN THEAD PLATES AND CATWALK SUPPORT CHANNELS AT SOME LOCATIONS. CATWALK TREAD PLATES HAVE RANDOM HEAVY RUST, HEAVY SURFACE RUST AND/OR UP TO 3/16* DEEP SECTION LOSS AT SOME LOCATIONS. F RANDOM HAND RAILS HAVE UP TO 1* BENDS. SKETCH 8	RUST W/ NEGLIGIBLE TO 1/16	" DEEP SECTION LOS	SS. RIVET HEADS	W/ UP TO 40% SECTION L	OSS AT SOME OF	THESE LO	CATIONS.	E LAMINATED
REVISION A DATE: 9/10/2018 CREW: MAR, MSO, MJL REVISION A DATE: CREW:								
	REVISION A DATE: 9/10/	2018 CRE W: MAR, M	ISO, MJL	REVISION A	DATE:	CREW:		SKETCH 8
						CREW:		

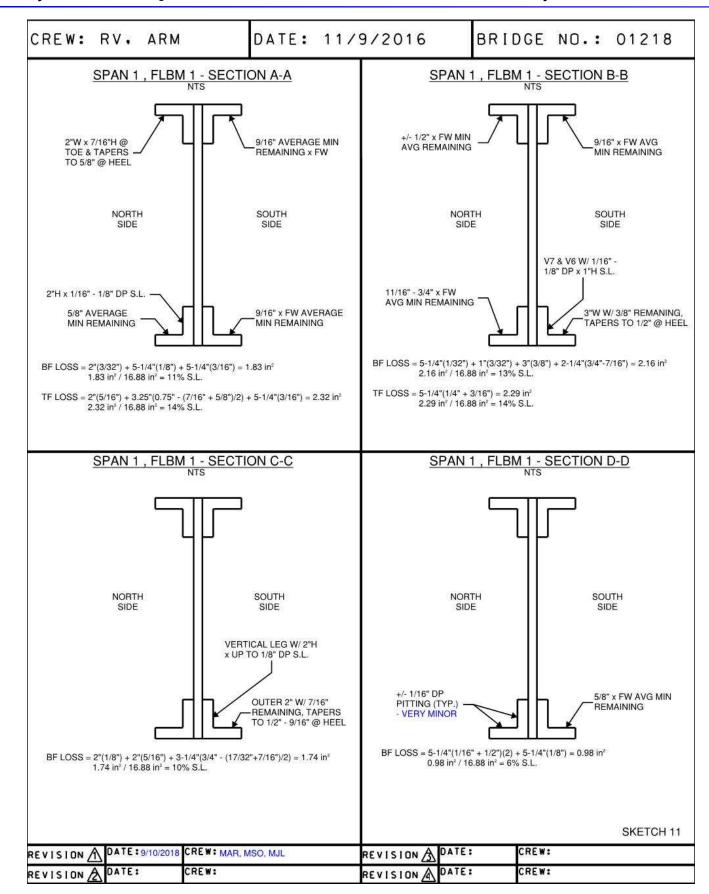
Bridge No: 01218

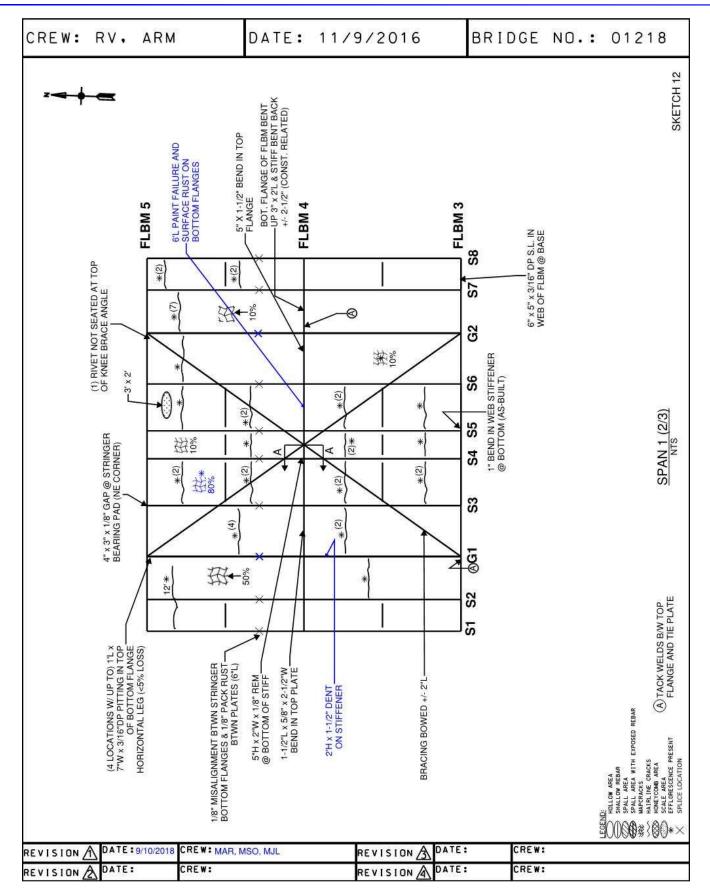


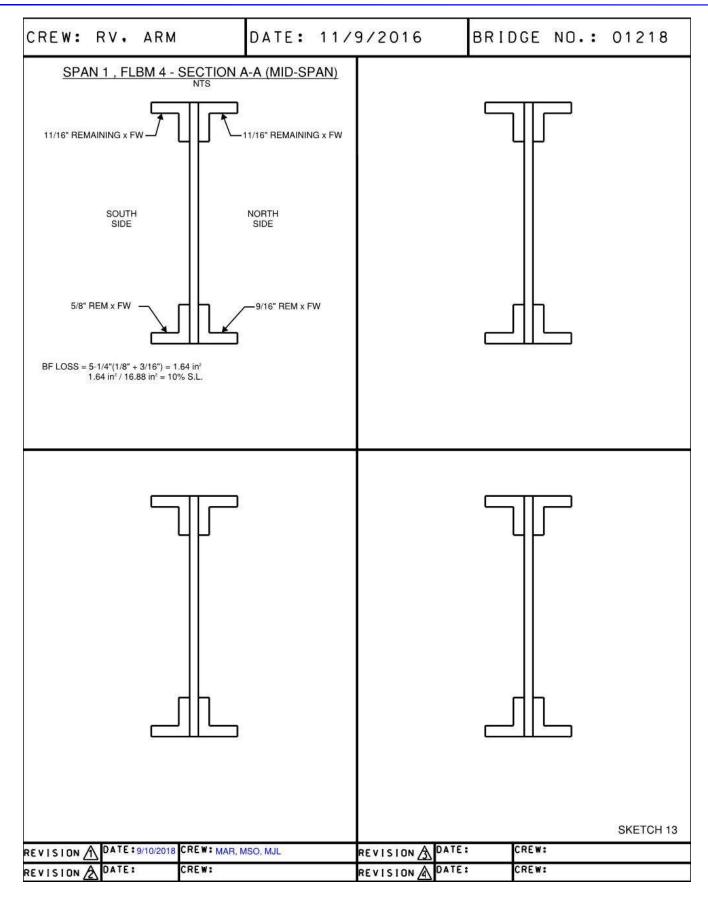
Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

DATE: CREW: BRIDGE NO.: 01218 RV. ARM 11/9/2016 TYPICAL FLOOR BEAM CROSS SECTION **SKETCH 10** S8 NORTH STIFFENER BENT 1/4"WEST FOR 6"H S7 3"W x 1/4" MIN. REMAINING @ S. BOTTOM FLANGE ANGLE HORIZONTAL LEG. GS ORIGINAL ANGLE: 6" x 3/4" = 8.46 in² CROSS SECTIONAL AREA 2" L x 1"H PERF. AT SOUTH WEB STIFFENER 1/4"© PERF. AT BASE OF SOUTH STIFFENER S6 TOP FLANGE NORTH ANGLE OF HORIZONTAL LEG BENT DOWN 5/8" x 6"L FOLLOWING SHEET (LAM. RUST & SECTION LOSS IS WORST @ THE JOINT SIDE OF THE FLOORBEAMS) BOTTOM 3" OF STIFFENERS & S. ELEVATION W/ HEAVY SECTION LOSS @ OUTSTANDING LEGS, UP TO TOP FLANGE & BOTTOM FLANGE ANGLES W/ LAM. RUST & SECTION LOSS. SEE CROSS SECTIONS ON SPAN 1, FLBM 1 NTS S5 $\overline{\mathbf{A}}$ S4 1/2"ø PERF. AT BASE OF SOUTH STIFFENER - FLANGE LOSS ONLY SHOWN IF GREATER THAN OR EQUAL TO 5% (5% \times 16.88 in² = 0.84 in² -FLANGE ANGLE AREA = 6" x 6" x 3/4" ANGLE = 8.44 in² x (2) = 16.88 in² WEBS = 66" x 3/8" (NO LOSSES IN CRITICAL ZONES @ INTERIOR) TOP FLANGE HORIZONTAL LEGS (N. & S.) W/ RAND. MINOR BENDS ASSUME FW / FH LOSSES = 5-1/4" WIDE / HIGH @ FLANGES ; RANDOM 1/16" SECTION LOSS THROUGH-OUT @ FLANGE. S3 o INTERIOR PORTIONS OF FLBMS S. BOTTOM FLANGE ANGLE HORIZONTAL LEG W/ 7/16" MIN REMAINING x 3"W (WORST NEAR S3; 9/16" @ MID PANEL) 5 KNIFE EDGE W/ PERFS UP TO 1"H. S2 GENERAL NOTES: S DATE: CREW: DATE: 9/10/2018 REVISION A CREW: MAR, MSO, MJL REVISION DATE: CREW: DATE: CREW: REVISION REVISION

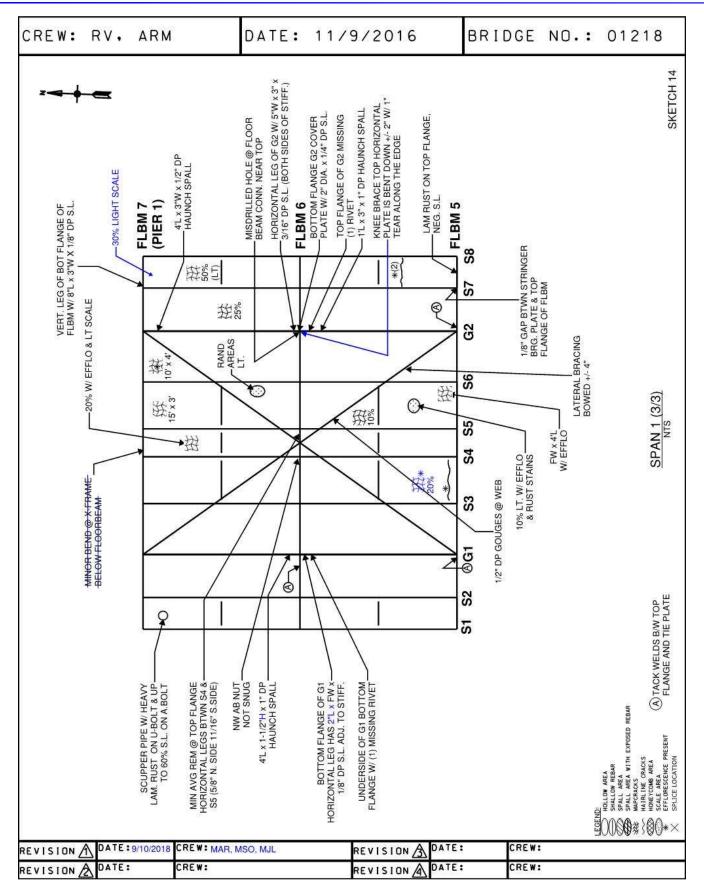
Bridge No: 01218

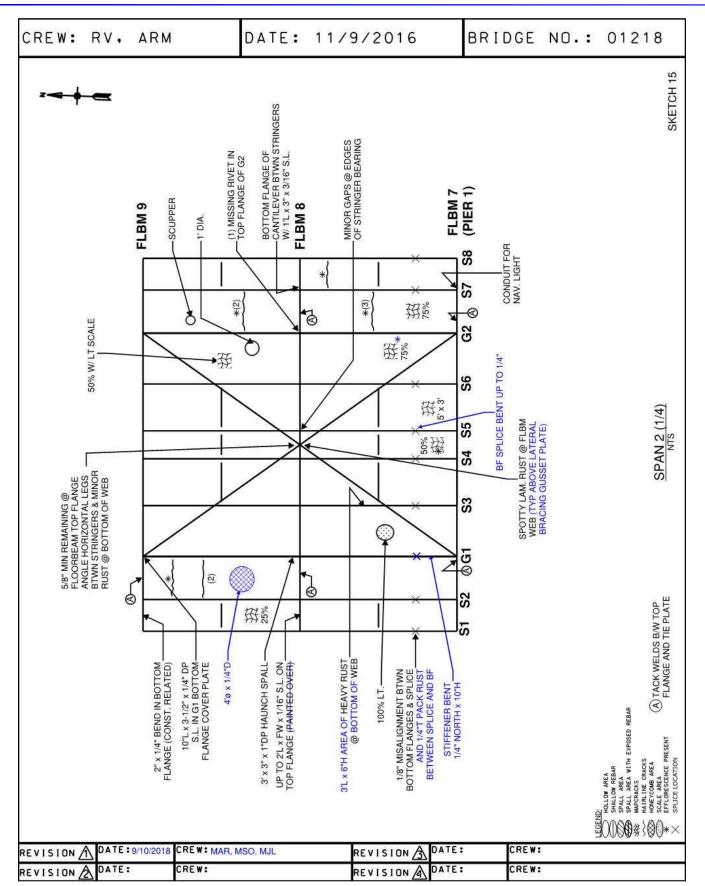


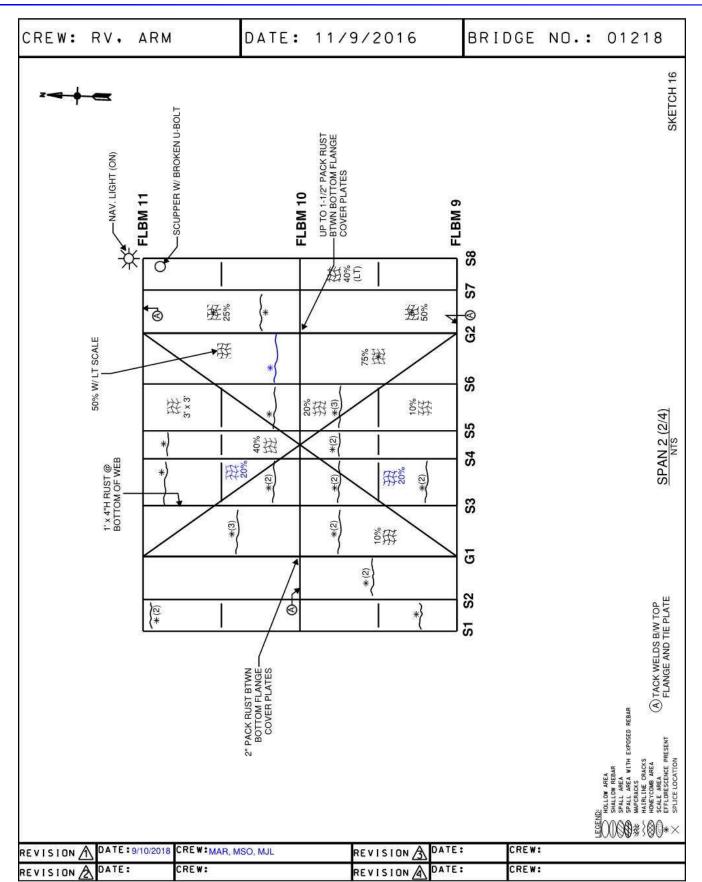




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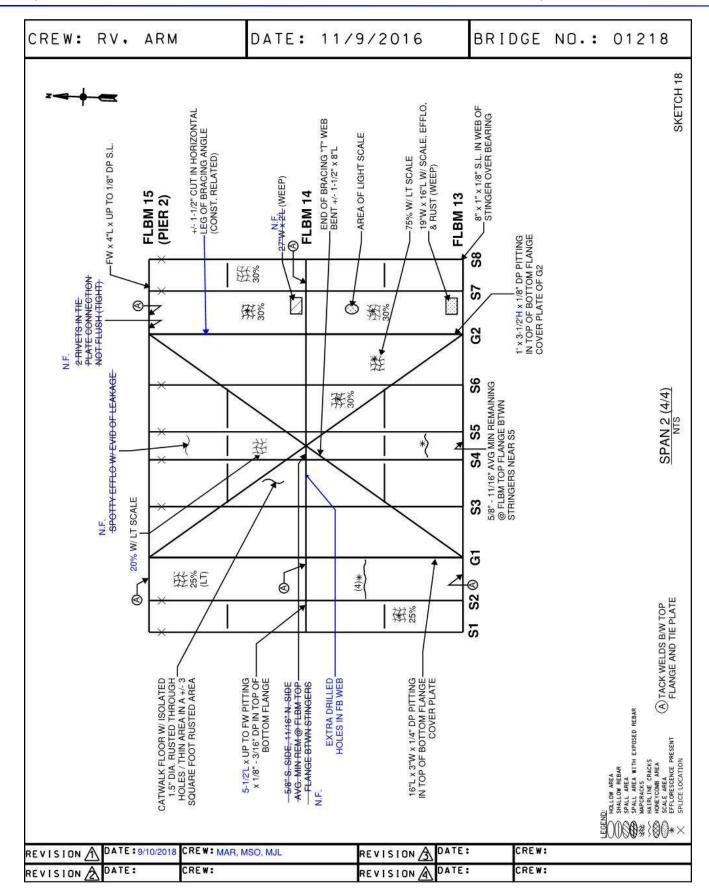




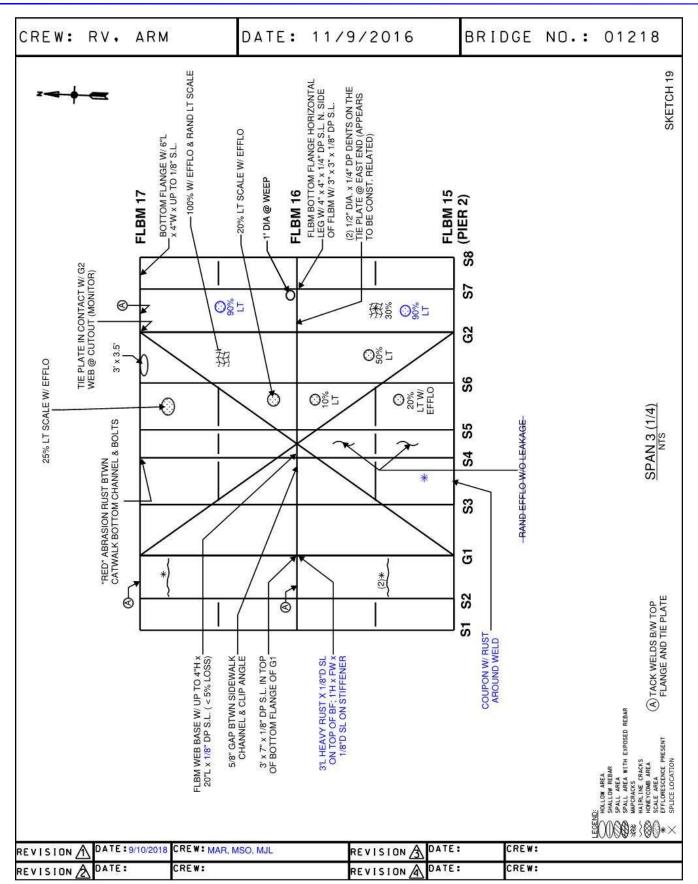
Bridge No: 01218

 	CREW: RV,	ARM		9/2016	BRIDGE	NO.:	State.
2L x 3W x 1H ² DO NTOPOF BOTTOM FLANCE COVER PLATE BOTTOM FLANCE COVER PLATE BOTTOM FLANCE COVER PLATE (4) MISSING RIVETS IN GIRDER (4) MISSING RIVETS IN GIRDER		Here and the second sec	(4) MISSING F GIRDER WEB 2° DIA x 3/1 BOTTOM FL BOTTOM FL FLBM 12 FL x FW		G2 S7 S8 S.L. IN TOP OF BF OF G2 W/ FW x 3" x 3/16" DP S.L. & RIVET HAS 50% S.L.		SKETCH 17
(4) MISSING RIVETS IN GIRDER WEB SPLICE (2) MISSING RIVETS IN GIRDER WEB SPLICE (2) MISSING RIVETS IN GIRDER (4) MISSING RIVETS IN G	5	(3)		\rightarrow	61		SPAN 2 (3/4) NTS
2" DIA. > WEB OF F SPLUM SPLUM MIRIN MIRIN	2'L × 3"W × 1/4" I BOTTOM FLANC		B SPLICE. HOLES PLATE IN COTWIGI	BLL X THX 1/16"D S.L. @ BOT OF WEB	Ś		REB
		(4)	2" DIA x 1/			: UN E	

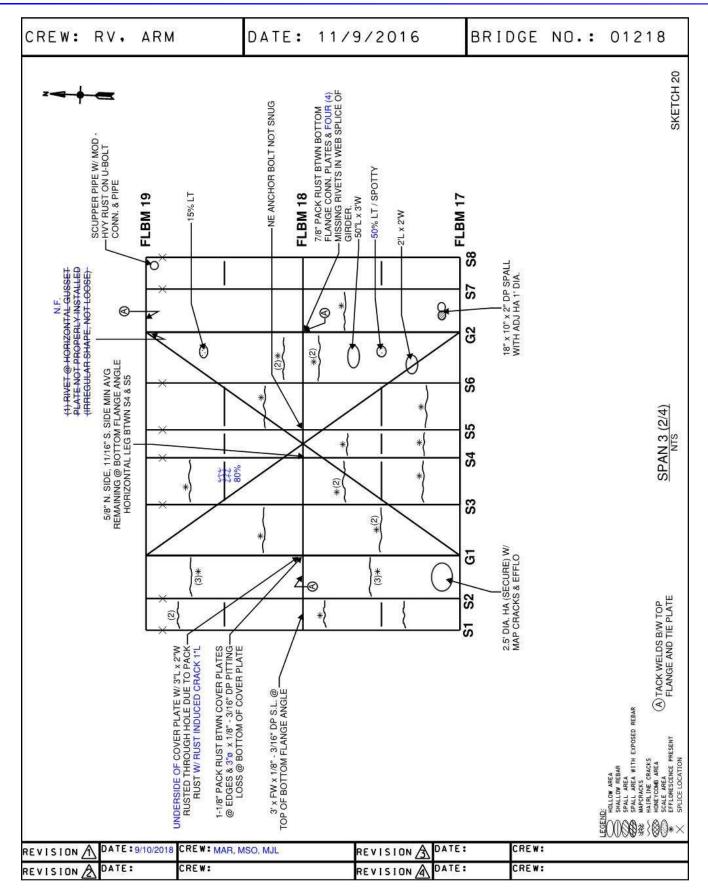
Bridge No: 01218



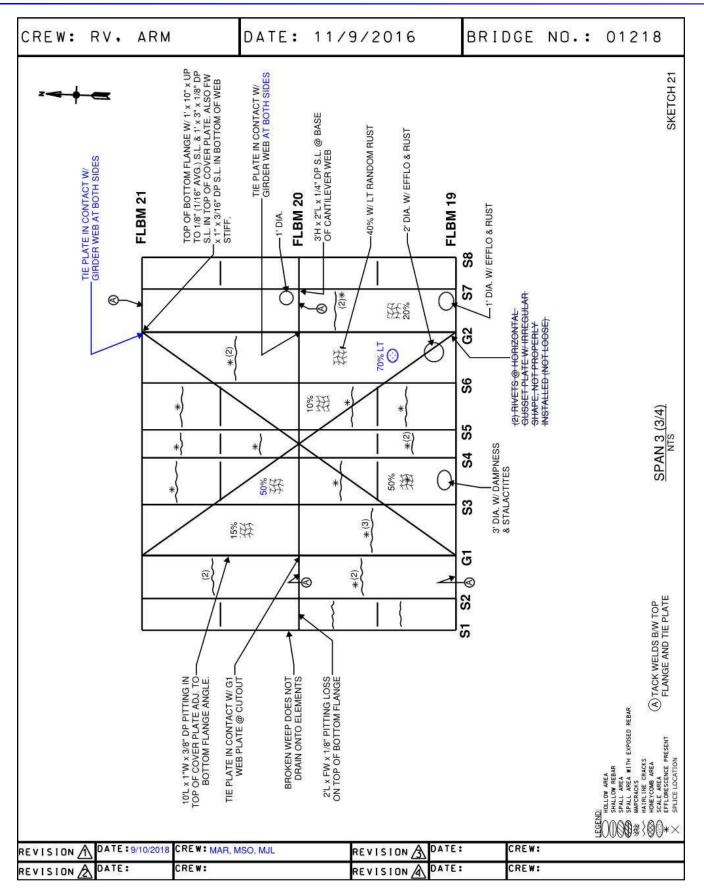
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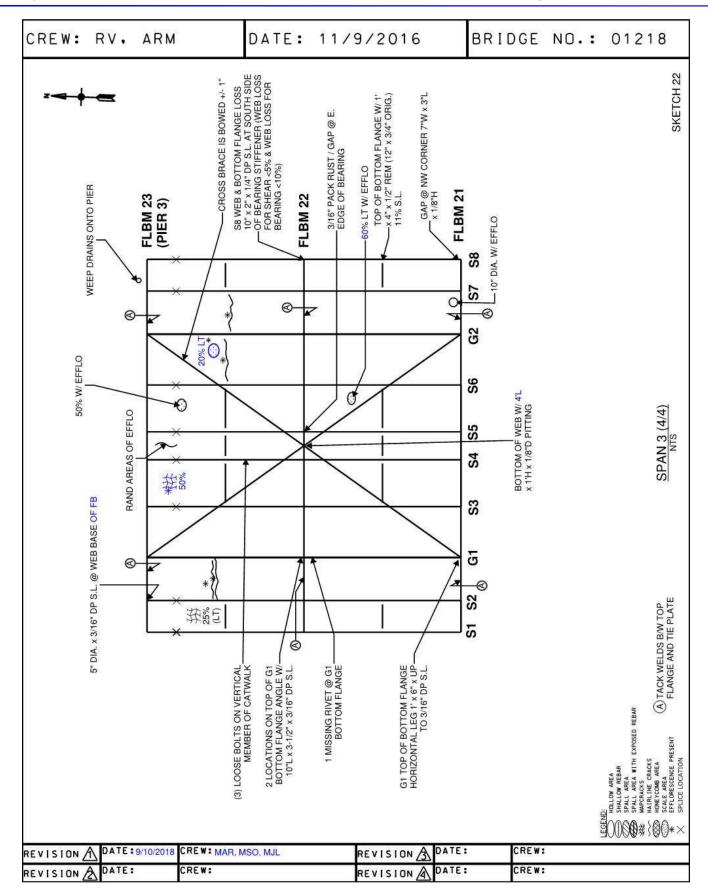


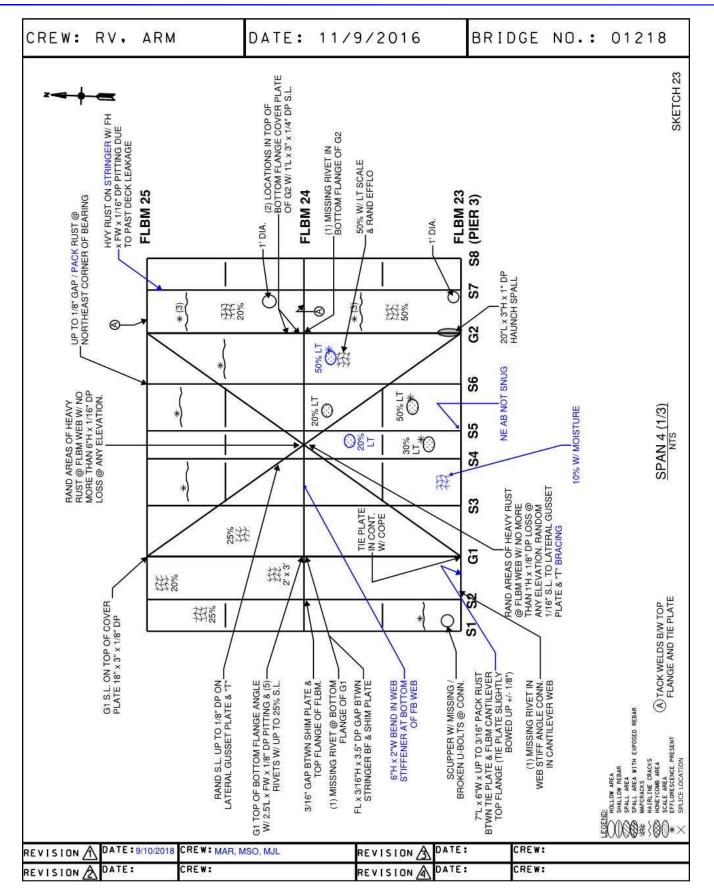
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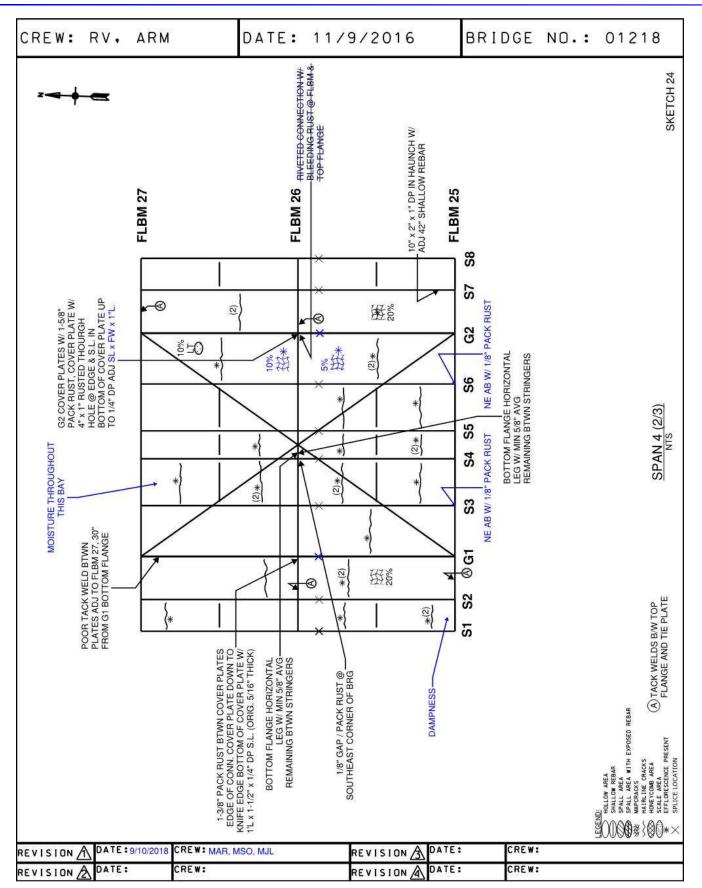


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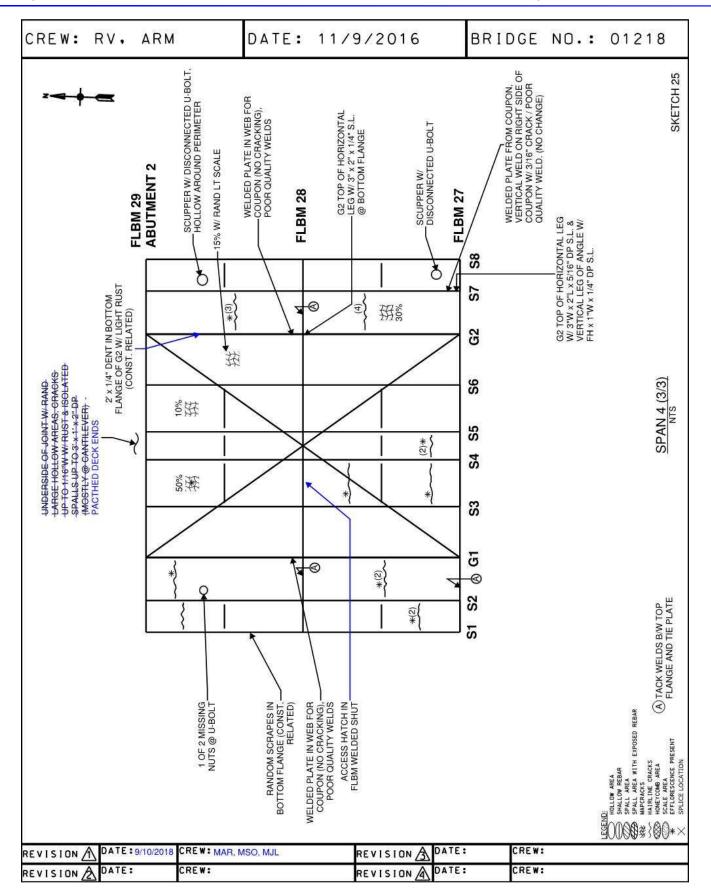


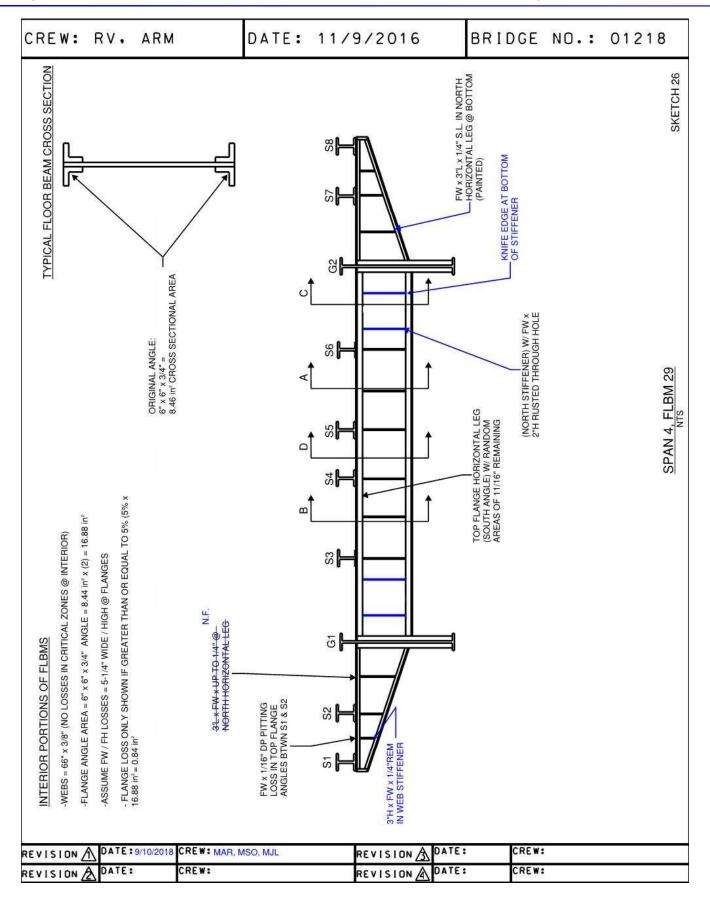


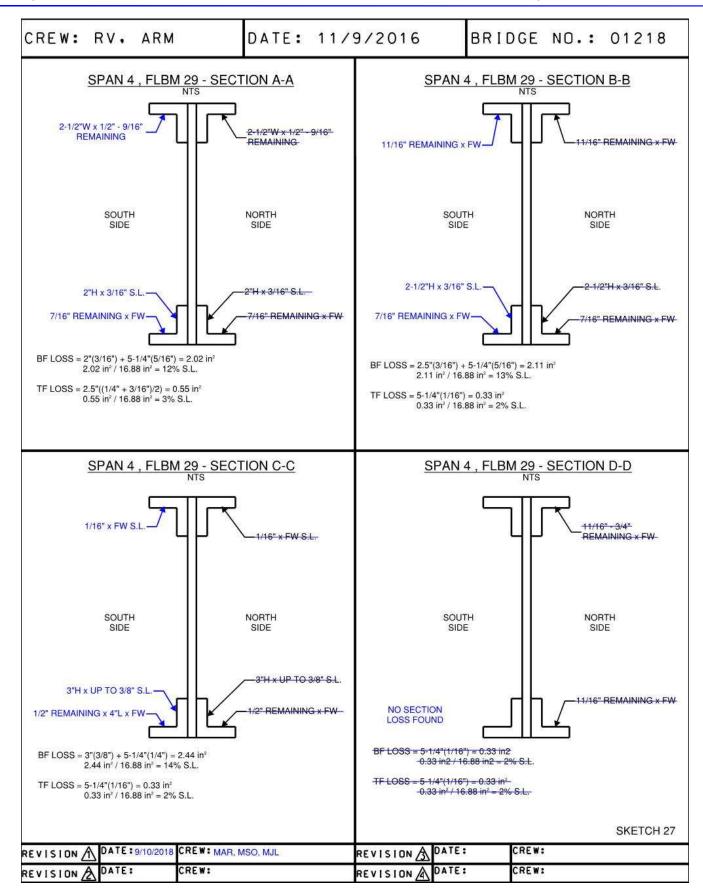




Bridge No: 01218







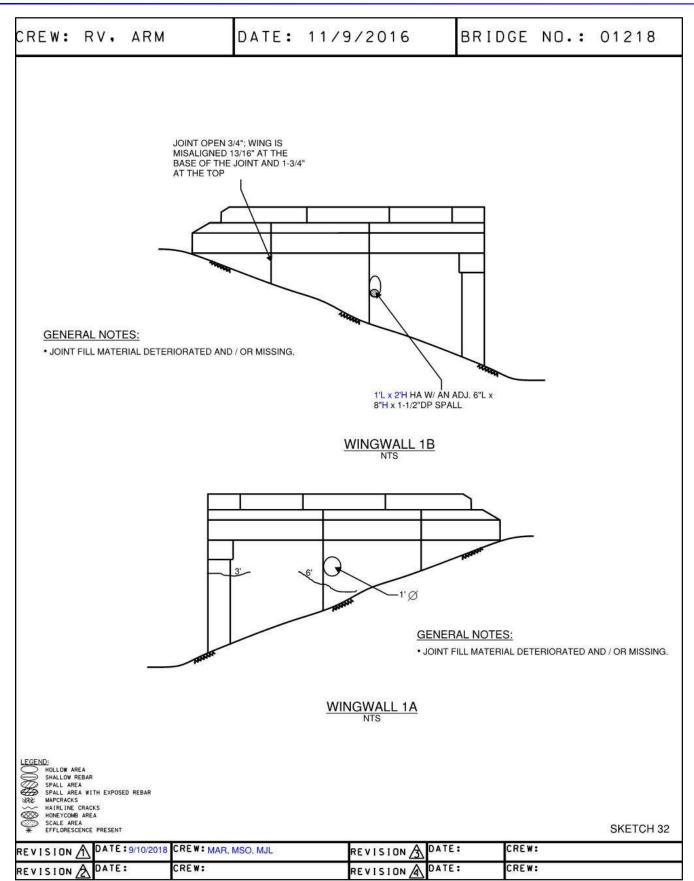
Town: NEWTOWN **Carried:** INTERSTATE 84 EASTBOUND **Crossed:** HOUSATONIC RIVER Inventory Route: NHS

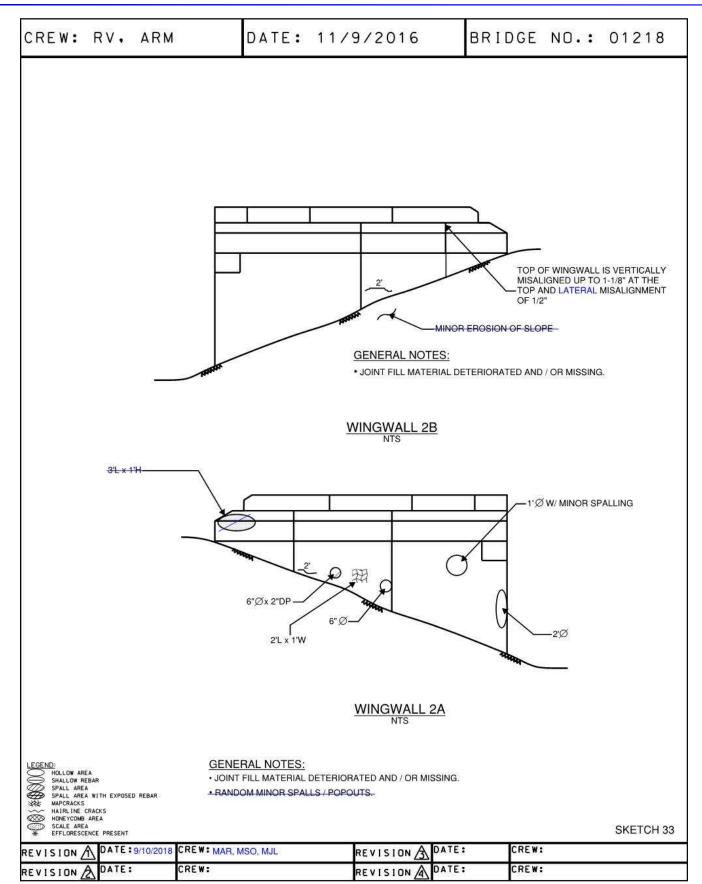
REW:	RV, ARM	t .	DATE:	11/9/2016	BRIDGE	NO.:	0121
OTHER FL	BMS (INTERIOR PC	RTIONS)					
	5" x 3/8" (NO LOSSE		ONES @ INTERI	OR)			
	NGLE AREA = 6" x						
	FW / FH LOSSES						
				TO 5% (5% x 16.88 in ² = 0.84 i	n²)		
	MIDSPAN OF FLBM				0.040		
			16.88 in² = 10% B	F LOSS (TF < 5% LOSS)			
	MIDSPAN OF FLB						
			16.88 in² = 6% tF I	_OSS (BF < 5% LOSS)			
	MIDSPAN OF FLB						
			16.88 in ² = 6% TF	LOSS (BF < 5% LOSS)			
	MIDSPAN OF FLB						
			16.88 in ² = 6% BF	LOSS (TF < 5% LOSS)			
	MIDSPAN OF FLB						
			16.88 in² = 6% BF	LOSS (TF < 5% LOSS)			
							SKETCI
VISION A	DATE: 9/10/2018	CREW: MAR, M	ISO, MJL		DATE: CREW	•	
VISION A	DATE:	CREW:		REVISION	DATE: CREW	:	

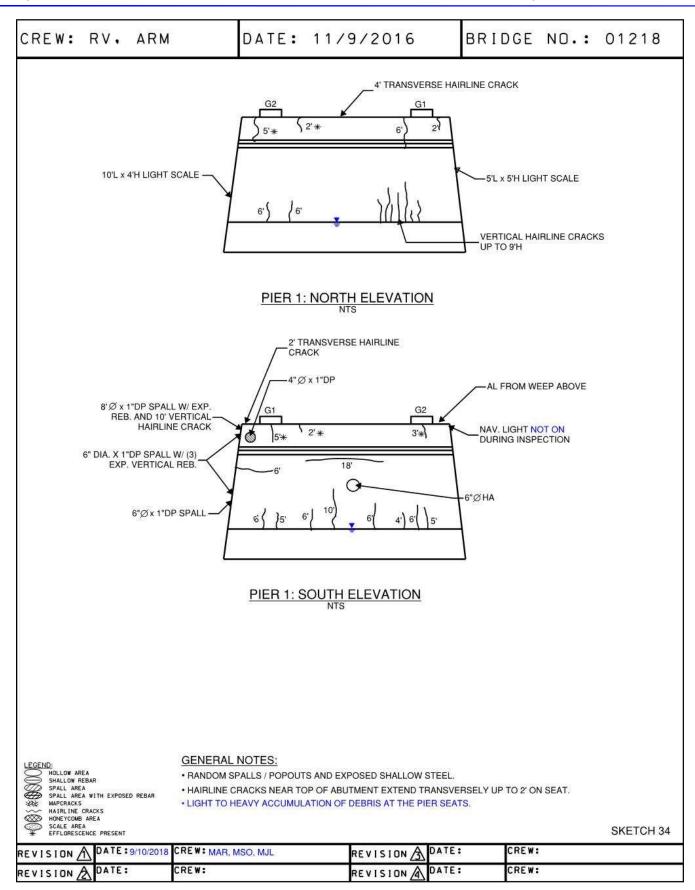
WEBS = 3/ FLANGE A - FLANGE - ALSO WI -LOSS AT E -% S.L. = (0) FLBM 8 EA LOSS AT E % S.L. = (0) FLBM 14 W LOSS AT E % S.L. = (0) FLBM 16 E LOSS AT E % S.L. = (1) FLBM 18 W LOSS AT E % S.L. = (0)		CRITICAL ZONE 5" x 4" x 1/2" ANG NN IF GREATER ARE SHOWN IF 	GLES = 4.75 in ² x (; R THAN 5% (5% x 9 GREATER THAN N.F. * x 1/8" = 0.69 in² in ²	9.5 in² = 0.48 in²) & IN CRITIC OR EQUAL TO 15%	CAL ZONES ; HORIZON	ITAL LEG IS 6"	
FLANGE A - FLANGE - ALSO WI -LOSS AT E -% S.L. = (0) FLBM 8 EA LOSS AT E % S.L. = (0) FLBM 16 E LOSS AT E % S.L. = (1) FLBM 18 W LOSS AT E % S.L. = (0)	NGLE AREA = (2) 6 LOSS ONLY SHOW EB LOSSES ONLY EST CANTILEVER 3TM FLANGE: FW : 0.69/9.5) × 100 – 7% AST CANTILEVER 3TM FLANGE: 3" × 3 0.56/9.5) × 100 = 6% VEST CANTILEVER 3TM FLANGE: FW × 0.86/9.5) × 100 = 9% CAST CANTILEVER 3TM FLANGE: 4"(1/	5" x 4" x 1/2" ANG WN IF GREATER ARE SHOWN IF - - - - - 3/16" S.L. = 5.5" - - - - - - - - - - - - - - - - - - -	GLES = 4.75 in ² x (; R THAN 5% (5% x 9 GREATER THAN N.F. * x 1/8" = 0.69 in² in ²	9.5 in² = 0.48 in²) & IN CRITIC OR EQUAL TO 15%	CAL ZONES ; HORIZON	ITAL LEG IS 6"	
- FLANGE - ALSO WI - LOSS AT E - % S.L. = (0) FLBM 8 EA LOSS AT E % S.L. = (0) FLBM 14 W LOSS AT E % S.L. = (0) FLBM 16 E LOSS AT E % S.L. = (1) FLBM 18 W LOSS AT E % S.L. = (0)	ELOSS ONLY SHOW EB LOSSES ONLY EST CANTILEVER BTM FLANGE: FW 0.69/9.5) × 100 = 7% AST CANTILEVER BTM FLANGE: 3" × 3 0.56/9.5) × 100 = 6% VEST CANTILEVER BTM FLANGE: FW × 0.86/9.5) × 100 = 9% CAST CANTILEVER BTM FLANGE: 4"(1/	WN IF GREATER ARE SHOWN IF 	R THAN 5% (5% x 9 GREATER THAN N.F. 'x 1/8" = 0.69 in ²	9.5 in² = 0.48 in²) & IN CRITIC OR EQUAL TO 15%	CAL ZONES ; HORIZON	ITAL LEG IS 6"	
- ALSO WI - LOSS AT E - % S.L. = (0 FLBM 8 EA LOSS AT E % S.L. = (0 FLBM 14 V LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (0	EB LOSSES ONLY EST CANTILEVER 3TM FLANGE: FW 0.69/9.5) × 100 – 7% AST CANTILEVER 3TM FLANGE: 3" × 3 0.56/9.5) × 100 = 6% VEST CANTILEVER 3TM FLANGE: FW × 0.86/9.5) × 100 = 9% CAST CANTILEVER 3TM FLANGE: 4"(1/	ARE SHOWN IF + 1/8" S.L. = 5.5" 3/16" S.L. = 0.56 3 4 5/32" S.L. = 5.5"	GREATER THAN N.F. ' * 1/8" = 0.69 in[*] in [°]	OR EQUAL TO 15%	CAL ZONES ; HORIZON	ITAL LEG IS 6"	
-FLBM 4 WI -LOSS AT E -% S.L. = (0 FLBM 8 EA LOSS AT E % S.L. = (0 FLBM 14 V LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (0	EST CANTILEVER 3TM FLANGE: FW 3.56/9.5) × 100 = 7% AST CANTILEVER 3TM FLANGE: 3" × 3 0.56/9.5) × 100 = 6% VEST CANTILEVER 3TM FLANGE: FW × 0.86/9.5) × 100 = 9% CAST CANTILEVER 3TM FLANGE: 4"(1/	- x 1/8" S.L. = 5.5" 3/16" S.L. = 0.56 3 4 5/32" S.L. = 5.5'	N.F. ' x 1/8" = 0.69 in² in ²				
-LOSS AT E -% S.L. = (0 FLBM 8 EA LOSS AT E % S.L. = (0 FLBM 14 W LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 W LOSS AT E % S.L. = (0	BTM FLANGE: FW 0:69/0.5) × 100 = 7% AST CANTILEVER BTM FLANGE: 3" × 3 0:56/9.5) × 100 = 6% VEST CANTILEVER BTM FLANGE: FW × 0:86/9.5) × 100 = 9% CAST CANTILEVER BTM FLANGE: FW × 0:86/9.5) × 100 = 9% CAST CANTILEVER BTM FLANGE: 4"(1/2)	x 1/8" S.L. = 5.5" 3/16" S.L. = 0.56 3 4 5/32" S.L. = 5.5"	' x 1/8" = 0.69 in^a in ^a	2			
-% S.L. = (0 FLBM 8 EA LOSS AT E % S.L. = (0 FLBM 14 V LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (0	2.69/9.5) × 100 – 7% AST CANTILEVER 3TM FLANGE: 3" × 3 0.56/9.5) × 100 = 6% VEST CANTILEVER 3TM FLANGE: FW × 0.86/9.5) × 100 = 9% CAST CANTILEVER 3TM FLANGE: 4"(1/	3/16" S.L. = 0.56 3 4 5/32" S.L. = 5.5'	' x 1/8" = 0.69 in^a in ^a	2			
FLBM 8 EA LOSS AT E % S.L. = (0 FLBM 14 W LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 W LOSS AT E % S.L. = (0	AST CANTILEVER BTM FLANGE: 3" x 3 0.56/9.5) x 100 = 6% VEST CANTILEVER BTM FLANGE: FW x 0.86/9.5) x 100 = 9% CAST CANTILEVER BTM FLANGE: 4"(1/	3/16" S.L. = 0.56 3 4 5/32" S.L. = 5.5"		a			
LOSS AT E % S.L. = (0 FLBM 14 V LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (0	3TM FLANGE: 3" × 3 0.56/9.5) × 100 = 6% <u>VEST CANTILEVER</u> 3TM FLANGE: FW × 0.86/9.5) × 100 = 9% <u>CAST CANTILEVER</u> 3TM FLANGE: 4"(1/	<u>}</u> < 5/32" S.L. = 5.5'		a			
% S.L. = (0 FLBM 14 V LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (0	0.56/9.5) x 100 = 6% VEST CANTILEVER 3TM FLANGE: FW x 0.86/9.5) x 100 = 9% CAST CANTILEVER 3TM FLANGE: 4"(1/	<u>}</u> < 5/32" S.L. = 5.5'		a			
FLBM 14 W LOSS AT E % S.L. = (0 FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 W LOSS AT E % S.L. = (0	VEST CANTILEVER 3TM FLANGE: FW x 9.86/9.5) x 100 = 9% 3AST CANTILEVER 3TM FLANGE: 4"(1/	<u>8</u> x 5/32" S.L. = 5.5'	" x 5/32" = 0.86 in ¹	2			
LOSS AT E % S.L. = (0 <u>FLBM 16 E</u> LOSS AT E % S.L. = (1 <u>FLBM 18 V</u> LOSS AT E % S.L. = (0	3TM FLANGE: FW x 9.86/9.5) x 100 = 9% 2AST CANTILEVER 3TM FLANGE: 4"(1/	< 5/32" S.L. = 5.5'	" x 5/32" = 0.86 in'	2			
% S.L. = (0 <u>FLBM 16 E</u> LOSS AT E % S.L. = (1 <u>FLBM 18 W</u> LOSS AT E % S.L. = (0	0.86/9.5) x 100 = 9% AST CANTILEVER BTM FLANGE: 4*(1/		" x 5/32" = 0.86 in ³	2			
FLBM 16 E LOSS AT E % S.L. = (1 FLBM 18 V LOSS AT E % S.L. = (0	AST CANTILEVER						
LOSS AT E % S.L. = (1 <u>FLBM 18 W</u> LOSS AT E % S.L. = (0	3TM FLANGE: 4"(1/4						
% S.L. = (1 <u>FLBM 18 V</u> LOSS AT E % S.L. = (0		A"\ . 0"/4/0"\ 4					
<u>FLBM 18 V</u> LOSS AT E % S.L. = (0	.38/9.5) x 100 = 159	4)+3(1/0)=1	1.38 in²				
LOSS AT E % S.L. = (0		%					
% S.L. = (0	VEST CANTILEVER	3					
	BTM FLANGE: FW x	c 5/32" = 5.5" x 5/	/32" = 0.86 in ²				
FLBM 29 V	0.86/9.5) x 100 = 9%	2					
	VEST CANTILEVER	1					
TOP FLAN	GE - 5.5" x 3/16" S.	L. <u>– 1.03 in² –</u>					
% S.L. = (1	.03/9.5) = 11% -		DEEPEST SECT	FION LOSS			
BOTTOM	FLANGE - 5.5" x 1/4	" S.L. = 1.38 in²	FOUND TO BE 1 THIS LOCATION				
% S.L. = (1	.38/9.5) = 14% -						
FROM GEI	NERAL NOTES (SN	IOOPER):					
BOTTOM F	FLNG. LOSSES AD.	J. TO STIFF'S: 5.	.5" x 3/16" = 1.03 in	n²			
% S.L. = (1	.03/9.5) = 11%						
BOTTOM	LNG. BTWN STRIN	NGERS S7 & S8:	: (4" x 1/8") + (3.5"	x 1/8") = 0.94 in ²			
% S.L. = (0	0.94/9.5) = 10%						
							SKET
SION A	DATE: 9/10/2018			REVISION	A DATE:	CREW:	

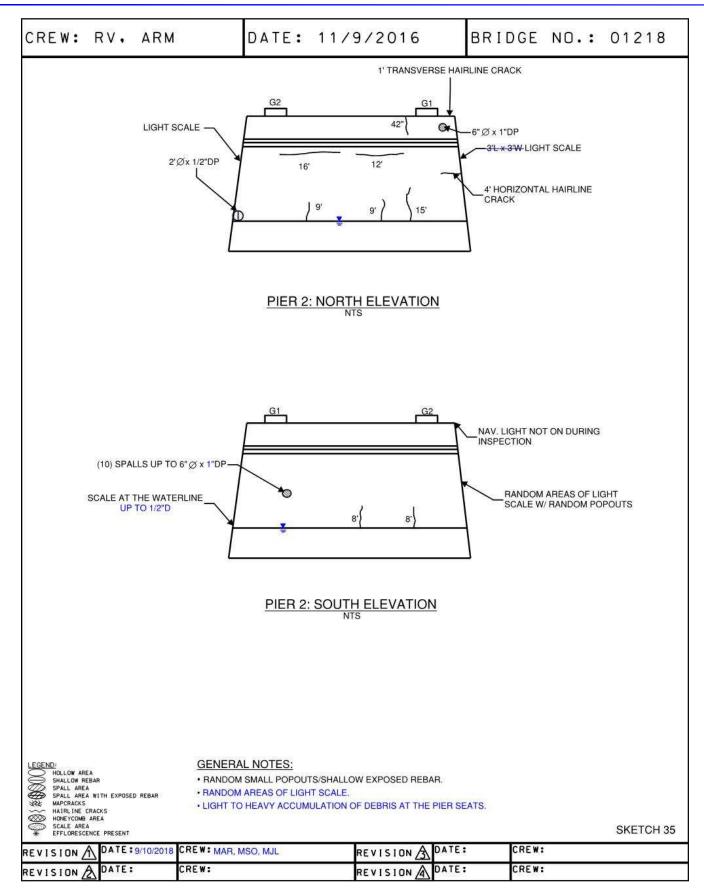
EVISION A DATE			REVISION	DATE:	CREW:		
		MAR, MSO, MJL		DATE:	CREW:	LEGEND: HOLLOW AREA SMALLOW REBAR SMALL AREA SPALL AREA SPALL AREA SPALL AREA MITH EXPOSED REBAR	HALPLINE CRACKS HALPLINE CRACKS HONEYCOMB AREA SOLLE AREA FEFLIDRECEME POPCENT
	UN-EVEN POUR OF BACKWALL W/ CONCRETE OVERPOUR AND EDGE SPALLS THROUGHOUT SPALLS THROUGHOUT 4-1/2L x 8"H x 2"DP	11L X 4'H X 1'DP 1'L X 4'H X 1'DP	G2 G1	DIRT / STONE SLOPE PROTECTION		GENERAL NOTES: • HEAVY GRAFFITI ON ABUTMENT STEM. • LIGHT TO MODERATE SCALE UP TO 1/4"D. • LIGHT TO HEAVY ACCUMULATION OF DEBRIS AT THE ABUTMENTS / PEDESTALS.	SOUTH ABUTMENT (1) SKFTCH 30
CREW: RV,	ARM	DATE: 11	/9/2016	BRI	DGE NO.	: 012	18

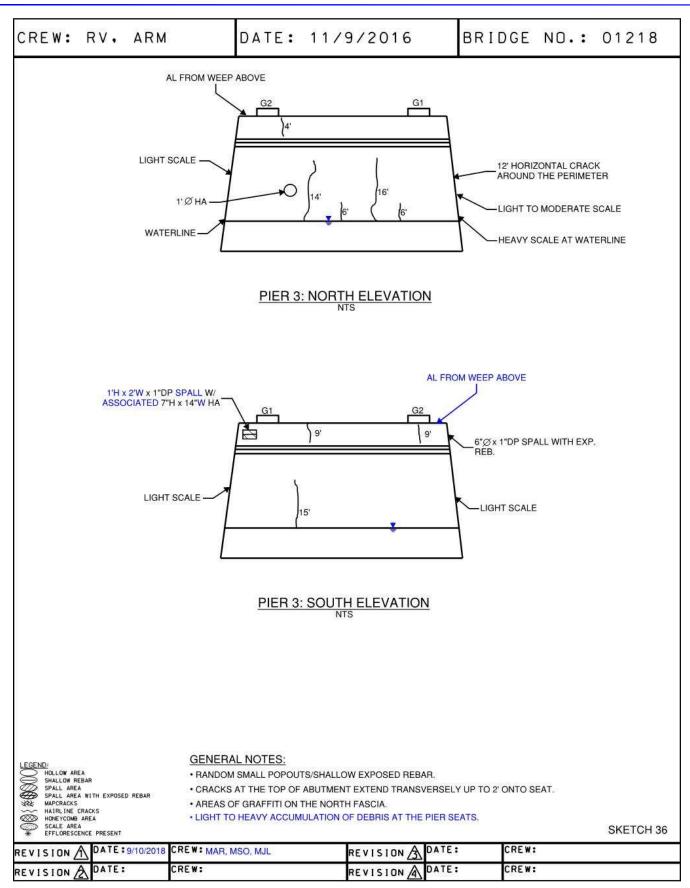
	UN-EVEN POUR OF BACKWALL W/ CONCRETE OVERPOUR AND EDGE SPALLS THROUGHOUT VCRETE PATCH		0			SKETCH 31
	UN-EVEN POUF CONCRETE OVI SPALLS T SPALLS T SPAL	SPALLS ALONG COLD JOINT	G		CECEND: CECEN	
REVISION A DATE: 9/1	10/2018 CREW: MAR, MSO,	MJL REV	ISION A DATE:	CREW:	Part 1949 et dons Somethin 1	











Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 1

Bridge Identification Number

<image>

Photo Number: 2

Left (West) Elevation

Photo Taken: 09/10/2018

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 3

Right (East) Elevation



Photo Number: 4

West Approach From Bridge

Photo Taken: 09/10/2018

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 5

Bridge From East Approach

Photo Taken: 09/10/2018



Photo Number: 6

Photo Taken: 09/10/2018

Span 1 Bituminous Concrete Overlay, Looking Southwest

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 7

Span 4 Bituminous Concrete Overlay

Note: 5' long x 3' wide map cracks.



Photo Number: 8

Span 2, Underside of Deck, Looking North

Photo Taken: 09/11/2018

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 9

Underside of Deck (Typical Scale)

Photo Taken: 09/11/2018



Photo Number: 10

Photo Taken: 09/11/2018

Span 1, Deck Underside, West Overhang at Floor Beam 6 Note: Haunch spall.

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 11

Right Sidewalk / Fence, Looking North

Photo Number: 12

Right Sidewalk, Span 4

Photo Taken: 09/10/2018

Note: Hollow area 76" long x 43" wide with scale 20" long x 12" wide x 1-1/2" diameter.

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 13

Sidewalk Scupper Grate in Span 4

Note: Clogged.



Photo Number: 14

Left Parapet / Railing, Looking Southwest

Photo Taken: 09/10/2018

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 15

Left Parapet, Span 4 at Pier 3

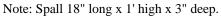




Photo Number: 16

Left Parapet, Span 2 at Midspan

Photo Taken: 09/10/2018

Note: Spall 18" long x 3" high x 1" deep and hollow area 10" diameter at sign connection.

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 17

Right Fence Base, Span 1



Note: Several spalls up to 8" high x 3' long x 1-1/2" diameter.

Photo Number: 18

Photo Taken: 09/10/2018

Right Rail Base at West Approach

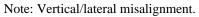
Note: Adjacent sections with 1" lateral misalignment.

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 19

Wingwall 2B



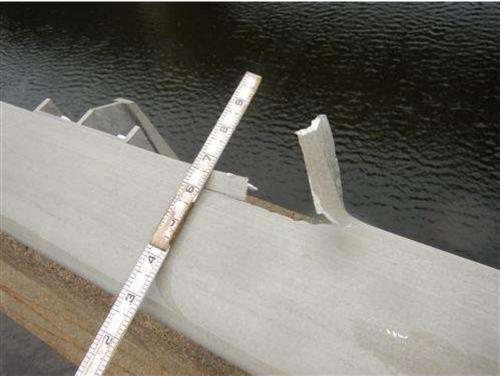


Photo Number: 20

Photo Taken: 09/10/2018

Left Railing, Span 2 Near Pier 1

Note: Tear 3" long x 1" wide.

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 21

Left Railing at Abutment 2

Note: 1 of 2 anchor bolts missing at end.



Photo Number: 22

Photo Taken: 09/10/2018

Right Fence at Abutment 2

Photo Taken: 09/10/2018



Photo Number: 23

Photo Taken: 09/11/2018

Scupper Adjacent to Stringer 1 Between Floor Beam 23 and 24

Note: Broken u-bolts at connection.



Photo Number: 24

Scupper Drain, Span 4, Right Overhang

Photo Taken: 09/10/2018

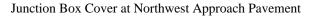
Note: Disconnected U-bolt.

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 25

Photo Taken: 09/10/2018



Note: Cracked/dented with 2 missing screws.



Photo Number: 26

Abutment 1 Compression Joint Seal with Concrete Headers, Looking West

Photo Taken: 09/10/2018



Photo Number: 27

Photo Taken: 09/10/2018

Abutment 2 Compression Joint Seal with Concrete Headers, Looking West



Photo Number: 28

Northwest Approach Guide Rail

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS



Photo Number: 29

Northwest Guide Rail



Note: 1 anchor bolt nut backed off 3/4" at parapet connection.

Photo Number: 30

Right Parapet at Abutment 1

Photo Taken: 09/10/2018

Note: Spall 10" long x 10" wide x 1-1/2" deep on top face.



Photo Number: 31

East Approach Pavement, Looking Northwest

Photo Taken: 09/10/2018



Photo Number: 32

Girder 1 Rocker Bearing at Abutment 1

Note: Anchor bolt nuts with section loss.

Photo Taken: 09/10/2018

Town: NEWTOWN Carried: INTERSTATE 84 EASTBOUND Crossed: HOUSATONIC RIVER Inventory Route: NHS

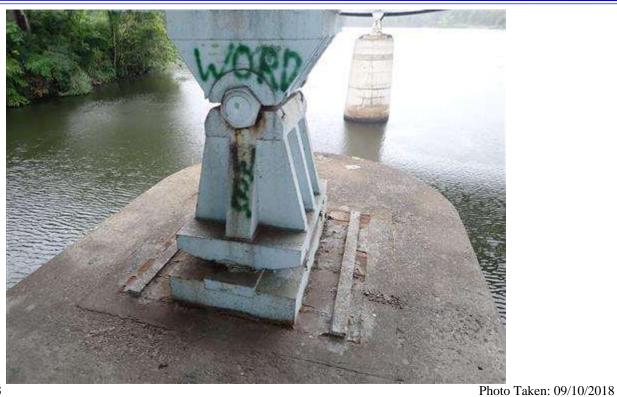


Photo Number: 33

Girder 1 Rocker Bearing at Pier 1



Photo Number: 34

Girder 1 Fixed Bearing at Pier 2



Photo Number: 35

Brackets Torch Cut Off Bearings

Photo Taken: 09/10/2018



Photo Number: 36

Girder 2 Expansion Bearing at Abutment 2

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Photo Number: 37

Span 2, Typical Stringer "Bearing" on Floor Beam



Photo Number: 38

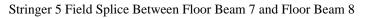
Stringer (Typical)

Photo Taken: 09/11/2018



Photo Number: 39

Photo Taken: 09/10/2018



Note: Bent up to 1/4".



Photo Number: 40

Inside Face of Girder (Typical)

Photo Taken: 09/11/2018

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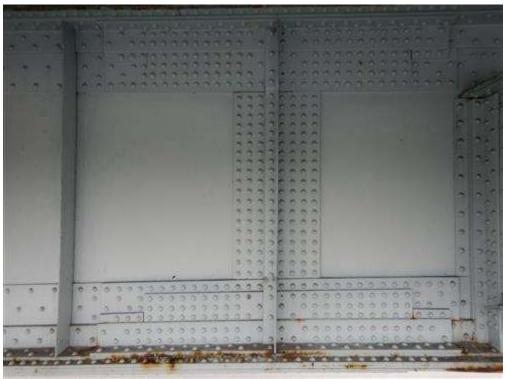


Photo Number: 41

Photo Taken: 09/11/2018

Girder 1 West Face at Floor Beam 12

Note: 4 missing rivets at splice.



Photo Number: 42

Girder 1 West Face at Floor Beam 6

Photo Taken: 09/11/2018

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Photo Number: 43

Girder 1 Floor Beam 6

Photo Number: 44

Girder 1 West Face at Floor Beam 10

Note: Pack rust between cover plates 2" thick.

Photo Taken: 09/11/2018

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Photo Number: 45

Girder 1 West Face at Floor Beam 13

Note: Bottom flange cover plate section loss.



Photo Number: 46

Girder 2, East Face at Floor Beam 21

Photo Taken: 09/11/2018

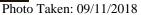
Note: Tie plate touching web.

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Photo Number: 47

Girder 1 Splice Plate Near Floor Beam 18



Note: 1-1/8" pack rust between cover plates at edges with up to 3/16" deep pitting loss at bottom of cover plate.



Photo Number: 48

Girder 1 Splice Plate North of Floor Beam 18

Photo Taken: 09/11/2018

Note: Underside has a 3" long x 2" wide rusted through hole due to pack rust.

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Photo Number: 49

Girder 2 East Face at Floor Beam 21

Note: Bottom flange section loss.



Photo Number: 50

Floor Beam (Typical)

Photo Taken: 09/11/2018



Photo Number: 51

Left Cantilever at Floor Beam 29 (Typical)

Photo Taken: 09/11/2018



Photo Number: 52

Floor Beam 1, South Face, Under Stringer 4

Photo Taken: 09/10/2018

Note: Section loss.

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Photo Number: 53

Photo Taken: 09/10/2018

Floor Beam 1, North Face, Under Stringer 7

Note: Web section loss 2" diameter x 1/4" diameter with pinhole at base.



Photo Number: 54

Photo Taken: 09/10/2018

Floor Beam 1, North Face under Stringer 1

Note: Stiff with rust holes full width x 1" high at top and bottom.



Photo Number: 55

Floor Beam 1, South Face, Between Girder 2 and Stringer 6.

Note: Top flange and stiffener bent.



Photo Number: 56

Floor Beam 3 South Side at West Cantilever Web

Photo Taken: 09/11/2018

Note: Heavy laminar rust with pitting adjacent to stiffeners.



Photo Number: 57

Photo Taken: 09/11/2018

Floor Beam 3, South Riveted End Plate

Note: Pack rust.



Photo Number: 58

Floor Beam 4, Top Flange Between Girder 1 and Stringer 3

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Photo Number: 59

Floor Beam 6 Connection to Girder 2

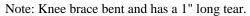




Photo Number: 60

Floor Beam 15 Between Stringer 3 and Stringer 4

Photo Taken: 09/10/2018

Note: No crack found in web coupon.

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Photo Number: 61

Photo Taken: 09/10/2018

Floor Beam 22 South Face Between Stringer 4 and Stringer 5

Note: Up to 1/8" deep section loss.



Photo Number: 62

Floor Beam 29, North Face Between Girder 2 and Stringer 6

Photo Taken: 09/10/2018

Note: Stiffener rust hole.



Photo Number: 63

Photo Taken: 09/10/2018

Floor Beam 29, North Face Between Stringer 1 and Stringer 2

Note: Top flange section loss.



Photo Number: 64

Floor Beam 29 North Face of Bottom Flange

Photo Taken: 09/11/2018

Note: Up to 3/16" deep section loss.



Photo Number: 65

Photo Taken: 09/10/2018

Lateral Bracing at Floor Beam 3 at Girder 1

Note: Bowed 1' long x 2-1/2" high.



Photo Number: 66

View of Catwalk (Typical)



Photo Number: 67

Photo Taken: 09/10/2018



Catwalk Connection to Floor Beam 16 North Side

Photo Number: 68

Abutment 1 Elevation



Photo Number: 69

Abutment 2 Elevation



Photo Number: 70

Note: Spalls along cold joint up to 2" deep.

Abutment 1



Photo Number: 71

Abutment 2, West Cheekwall

Note: Spall.

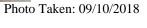




Photo Number: 72

Wingwall 1A Elevation



Photo Number: 73

Wingwall 2A

Note: Lateral misalignment.



Photo Number: 74

Photo Taken: 09/10/2018

Wingwall 1B Elevation

Note: Spall/hollow area.

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Photo Number: 75

Pier 1, South Elevation



Photo Number: 76

Channel Looking Downstream (East)

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Photo Number: 77

Channel Looking Upstream (West)